## NEW W. EDWARD BALMER ELEMENTARY SCHOOL

## TOWN OF NORTHBRIDGE, MASSACHUSETTS



## PLANNING BOARD SUBMISSION April 9, 2019

260 Merrimac St. Bldg. 7, $2^{\text {nd }}$ Floor Newburyport, Massachusetts 01950

Phone: 978-499-2999

Project Management
1000 Massachusetts Avenue
Cambridge, Massachusetts 02138
Phone: 617.547.5400
www.smma.com

212 Battery Street
Burlington, Vermont 05401
Phone: 802.863.1428
www.doreandwhittier.com

Mr. R. Gary Bechtholdt II, Town Planner<br>Town of Northbridge<br>Aldrich School Town Hall Annex<br>14 Hill Street<br>Whitinsville, MA 01588

## RE: W. Edward Balmer Elementary School - Planning Board Submission

## Dear Gary,

Please accept this submission to the Planning Board for the New Balmer Elementary School project, attached and delivered on this date. The project includes construction of a new Grades PK-5 elementary school on the site of the existing Balmer school, which will also involve the Vail Field parcel as part of the project.

We have attached our previously submitted Zoning Bylaws analysis, which provides some relevant information for the Planning Board. In addition, we have isolated some items of information requested in the Bylaws section 17349.1.E submission requirements that may not be included or easily inferred from the attached drawings, as follows:
(2)(e) The proposed school building is 167,352 gross square feet (GSF) in size.
(2)(h) The School Building Committee has proposed an electronic programmable LED sign at the front entrance of the school. It is intended to be mounted on (or recessed within) the masonry gateway shown on the site plan. A conceptual elevation drawing is attached. We are aware of the sign ordinance in the residential district, but would submit that a 12 SF internally lit, non-animated, white LED sign communicating activities and upcoming events at the school is both reasonable and essential for the school's function, and would not place an undue burden on neighbors.
(2)(k) Estimated earthwork is as follows:

Phase I (New Building Construction) Bulk Grade Cut: 21,850 CY; Bulk Grade Fill: 14,000 CY.
Phase II (West Parking Lot Construction) Bulk Grade Cut: 8,650 CY; Bulk Grade Fill: 1,420 CY.
(2)(p)[1] Traffic Impact Report, by Nitsch Engineering, dated January 26, 2018, attached. The report body is included; however, the appendix of some 163 pages containing the raw traffic count data is not, but is available upon request.

For the remainder of $(2)(p)[2-4]$ we would submit that these items are either covered under the Conservation Commission application or not germane to this public building project and should be waived for this application.

## ARCHITECTS

PROJECT MANAGERS
260 Merrimac Street Bldg 7
Newburyport, MA 01950
978.499.2999 ph
978.499.2944 fax

212 Battery Street
Burlington, VT 05401
802.863 .1428 ph
802.863.6955

Mr. Gary Bechtholdt, Town Planner
BALMER - Planning Submission
April 9, 2019
Page 2 of 2
Please contact me if you have any question on the above material, or require anything further. We look forward to working with you to continue the permitting process for this project.

Sincerely,
DORE \& WHITTIER ARCHITECTS, INC.
Architects - Project Managers


Tom Hengelsberg, AIA
Project Manager
Attachments
cc: File

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## SITE PLAN REVIEW

April 9, 2019
Applicant: Dore \& Whittier Architects, Inc.
Address: $\quad 260$ Merrimac Street, Building 7, Newburyport, MA 01950

## To the Planning Board of the Town of Northbridge

The undersigned, being the applicant for approval of a site plan shown on a plan entitled: "Planning Board Submission Package for the New W. Edward Balmer School" designed by Dore \& Whittier Architects/ Nitsch Engineering/ Horiuchi \& Solien Landscape Architects $\qquad$ , dated April 9,2019 and described as follows:

A plan showing $\qquad$
Civil Engineering: Demolition Plans, Roadway Layout Plans, Road Signage and Striping Plans, Building Location Plans, Site Grading Plans, Site Utility Plans, Site Drainage Plans, Selected Civil Engineering Profiles and Details.

Landscape Architecture: Overall Site Plan, Layout and Materials Plan and Enlargements, Planting Plans, Landscape Details.

Architectural: Building Elevations with materials called out, Colored 3-D Renderings, Signage Details
Electrical Engineering: Electrical Site Plan \& Details, Exterior Lighting Fixture Cut Sheets

Location: $\qquad$
Total acreage of tract: 30.08 acres (+-)
Total square footage of gross floor area proposed: $\qquad$
The project is a new structure or group of structures: Not Applicable $\underline{\mathbf{X} \quad \text { Yes }}$ $\qquad$ No

This project is an improvement, alteration, or addition to existing structures $\qquad$ Yes

X $\qquad$ No

Not Applicable

Said applicant hereby submits said site plan in accordance with the Northbridge Zoning By-law Article X $\S 173-49.1$ for approval of said site plan.

The undersigned's title to said land is derived from _ Whitin Machine Works
by deed dated $\qquad$ April 24, 1963 $\qquad$ and recorded in the $\qquad$ Worcester South $\qquad$
County District Registry of Deeds Book $\qquad$ 4369 Page 342 , registered in the $\qquad$
$\qquad$ County Registry District of the Land Court, Certificate of Title No. $\qquad$


Applicant's Address: Dore \& Whittier Architects, Tme., 260 Merrimac Street, Building 7, Newburyport, MA 01950
Applicant's Telephone:
$\qquad$ Date: 4.9.2019
Owner's Signature:
Owner's Address: : Town of Northbridge (for Northbridge Public Schools), 7. Main Street, Whitinsville, MA 01588

Owner's Telephone: $\qquad$ (NPS - 508-234-8156)

Applicant's Authorization if not the owner: $\qquad$

## Received by the Town Clerk:

Date: $\qquad$
Time: $\qquad$
Signature: $\qquad$
$\qquad$

## Instructions

The abutters list shall be prepared by the applicant and submitted to the assessor's office for certification.
Attach a copy or sketch of the most current assessor's plat showing the land described in this petition and the abutting parcels within three hundred feet ( $\mathbf{3 0 0}^{\boldsymbol{\prime}}$ ). Each parcel shall be numbered in accordance with the assessor's records.

Attach the completed list of the owners, from the most recent tax list, of each abutting parcel within three hundred feet ( $\mathbf{3 0 0}$ ') of a property line of the proposed subdivision.

## Plan Identification

| Project Name: New W. Edward Balmer Elementary School |
| :--- |
| Address of Project: $\quad 21$ Crescent Street, Whitinsville, MA 01588 |

Applicant: $\qquad$
$\qquad$
Address: $\qquad$
Telephone: $\qquad$

Owner: Town of Northbridge (for Northbridge Public Schools)
Address: 7 Main Street, Whitinsville, MA 01588
Telephone: $\quad 508-234-2095$ (NPS - 508-234-8156)

This is to certify that at the time of the last assessment for taxation made by the Town of Northbridge, the names and addresses of the parties assessed as adjoining owners to the parcel of land shown are written. This list is assumed to be complete to the best of our knowledge and belief.



## ABBUTTERS LISTING

NORTHBRIDGE, MA

| Map | Block | Lot | Unit | Owner~s Name | Co Owner~s Name | Address | City | ST | Zip | Parcel Location |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7 | 287 |  |  | AUStin luke C te | Holly l austin te | 325 SWIFT RD | whitinsvilile | MA | 01588 | 325 SWIFT RD |
| 7 | 288 |  |  | COLLINS RuSSELI D | KAThleen y collins | 292 MASON RD | whitinsvilie | MA | 01588 | 292 MASON RD |
| 7 | 289 |  |  | Chagnon irrevocable family trust | paul chagnon/denise zecco, trustee | 282 MASON RD | whitinsvilie | MA | 01588 | 282 MASON RD |
| 7 | 290 |  |  | Craig francine | C/O A PEREZ \& K RIVERA | 265 MASON RD | whitinsivile | MA | 01588 | 266 MASON RD |
| 7 | 291 |  |  | zANELLA PATRICK $T$ | tara a zanella | 254 MASON RD | whitinsvilie | MA | 01588 | 254 MASON RD |
| 7 | 292 |  |  | VALIS DAvid 0 | LINDA m VALIS | 244 MASON RD | whitinsvilie | MA | 01588 | 244 MASON RD |
| 7 | 293 |  |  | baris Charles R | CATHERINE F baris | 230 MASON RD | whitinsvilie | MA | 01588 | 230 MASON RD |
| 7 | 294 |  |  | COURTEMANCHE JOHN | JACQUELINE C COURTEMANCHE | 216 MASON RD | whitinsvilie | MA | 01588 | 216 MASON RD |
| 7 | 295 |  |  | ARMStrong john a | MARY L Armstrong | PO BOX 172 | whitinsvilie | MA | 01588 | 202 MASON RD |
| 7 | 296 |  |  | bigness kyle | keri l bigness | 192 MASON RD | whitinsville | MA | 01588 | 192 MASON RD |
| 7 | 297 |  |  | brooks John leroy |  | 178 MASON RD | whitinsvilie | MA | 01588 | 178 MASON RD |
| 7 | 298 |  |  | STOCKWELL EDWARD R SR | MARGARET B STOCKWELI, TE | 168 MASON RD | whitinsvilie | MA | 01588 | 168 MASON RD |
| 7 | 299 |  |  | kelleher sean D | C/O JEREMY HARRIS/LESLIE R COSGRO | 156 MASON RD | whitinsville | MA | 01588 | 156 MASON RD |
| 7 | 300 |  |  | Kourey nicholas w | C/O SCOTT \& SAMANTHA MURDOUGH | 146 MASON RD | whitinsville | MA | 01588 | 146 MASON RD |
| 7 | 301 |  |  | guglielmo kenneth r | maria a guglielmo | 138 MASON RD | whitinsville | MA | 01588 | 138 MASON RD |
| 7 | 302 |  |  | GAMbon thomas m |  | 130 MASON RD | whitinsivile | MA | 01588 | 130 MASON RD |
| 7 | 303 |  |  | SULLIVAN BRIAN J |  | 68 evergreen CR | whitinsvilue | MA | 01588 | 68 evergreen CR |
| 7 | 304 |  |  | fortin living trust | dennis $Ј$ \& BARBARA J Fortin, truste | 56 evergreen Cr | NORTHBRIDGE | MA | 01588 | 56 evergreen Cr |
| 7 | 306 |  |  | hay Craig d | MARY E HAY, te | 32 evergreen Cr | whitinsville | MA | 01588 | 32 evergreen cr |
| 7 | 307 |  |  | Cahalane Jonathan v | denise e cahalane | 20 evergreen cr | whitinsville | MA | 01588 | 20 evergreen Cr |
| 7 | 308 |  |  | neweli kenneta s | brenda l neweli | 19 evergreen cr | whitinsville | MA | 01588 | 19 evergreen cr |
| 7 | 309 |  |  | hawkes charles b | kristine b hawkes | 126 EAIRLAWN St | whitinsvilile | MA | 01588 | 126 FAIRLAWN St |
| 7 | 310 |  |  | froment kristine A | david ma froment, te | 31 evergreen CR | whitinsville | MA | 01588 | 31 evergreen cr |
| 7 | 311 |  |  | MALONE MIChael P | MELISSA A MALONE | 41 evergreen cr | Whitinsville | MA | 01588 | 41 evergreen cr |
| 7 | 312 |  |  | Cogliandro paul d | SUSAN M. COGLIANDRO | 53 evergreen cir | whitinsville | MA | 01588 | 53 evergreen cr |
| 7 | 313 |  |  | TOWN OF NORTHBRIDGE |  | N/A | whitinsville | MA | 01588 | evergreen cr |
| 7 | 314 |  |  | PILEGGI MARK \& DAvid pileggi jr. Tr | c/o Pileggi irrevocable trust | 65 evergreen CR | whitinsvilue | MA | 01588 | 65 evergreen Cr |
| 7 | 315 |  |  | vitagliano robert | elisabeth vitagliano | 94 MASON RD | whitinsvilile | MA | 01588 | 94 MASON RD |
| 7 | 316 |  |  | pileggi david J JR | Alison pileghi, Te | 120 MASON RD | whitinsvilue | MA | 01588 | 120 MASON RD |
| 7 | 317 |  |  | dembrowski stephen J | MARIE A DEmbrowski | 103 MASON RD | whitinsville | MA | 01588 | 103 MASON RD |
| 7 | 318 |  |  | gay bruce c | margaret m gay | 80 Dover Dr | whitinsvilue | MA | 01588 | 80 DOVER DR |
| 7 | 330 |  |  | DER MUGRDITCHIAN MARK | CYnthia der mugrditchian | 75 Dover DR | whitinsville | MA | 01588 | 75 Dover DR |
| 7 | 331 |  |  | fleming kevin J | C/O Philip \& SARAH HANNA | 89 Dover DR | whitinsville | MA | 01588 | 89 Dover DR |
| 7 | 332 |  |  | ROSSELLI ANTHONY J | C/O ANTHONY J ROSSELLI | 109 Dover DR | whitinsvilie | MA | 01588 | 109 Dover DR |
| 7 | 333 |  |  | CRAWFORD RYAN | CARRIE CRAWFORD, te | 115 MASON RD | whitinsville | MA | 01588 | 115 MASON RD |
| 7 | 334 |  |  | CASEY FAMILY NOMINEE TRUST | JOhn $T$ \& LOIS A CASEY TRS | 151 MASON RD | whitinsville | MA | 01588 | 151 MASON RD |
| 7 | 335 |  |  | SWARTZ Peter s | MARYANNE BELMONTE SWArtz | 96 Kerry Ln | whitinsville | MA | 01588 | 96 KERRY LN |
| 7 | 336 |  |  | miedema david ili \& Kathieen e, trs | miedema family living trust | 84 KERRY LN | whitinsvilue | MA | 01588 | 84 KERRY LN |
| 7 | 337 |  |  | Robinson daniel P |  | 72 KERRY LN | whitinsville | MA | 01588 | 72 KERRY LN |
| 7 | 347 |  |  | BOL NICHOLAS P | kelly s bol, te | 69 Kerry Ln | Whitinsville | MA | 01588 | 69 KERRY LN |
| 7 | 348 |  |  | durgin william R | LINDA F DURGIn | 81 kerry lane | whitinsvilue | MA | 01588 | 81 KERRY LN |
| 7 | 349 |  |  | OUILLEtte david u | maryann ouillette | 93 KERRY LN | whitinsville | MA | 01588 | 93 KERRY LN |
| 7 | 350 |  |  | barkley john C | beth a barkley | 175 MASON RD | whitinsville | MA | 01588 | 175 MASON RD |
| 7 | 351 |  |  | banning robert a | Elizabeth a banning | 191 MASON RD | whitinsville | MA | 01588 | 191 MASON RD |
| 7 | 352 |  |  | COOK BRIAN D | RATE E COOK, TE | 76 MIChael Ln | whitinsville | MA | 01588 | 76 MICHAEL LN |
| 7 | 353 |  |  | HENDERSON CHRISTOPHER | KAREN D henderson | 64 MICHAEL LN | WhITINSVILLE | MA | 01588 | 64 MICHAEL Ln |


| Map | Block Lot | Unit | Owner~s Name | Co Owner $\sim$ s Name | Address | City | ST | zip | Parcel Location |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7 | 354 |  | Townsend david J | JESSICA M TOWNSEND | 50 MICHAEL LN | whitinsville | MA | 01588 | 50 MICHAEL LIN |
| 7 | 361 |  | CALUORI MICHAEL JR | BARBARA WINSOR CALUORI | 45 michael lane | whitinsville | MA | 01588 | 45 MICHAEL LN |
| 7 | 362 |  | mutell robert a | CARolyn a mutell | 57 michael lane | whitinsville | MA | 01588 | 57 MICHAEL LN |
| 7 | 363 |  | hedtier ashley e | SCOTT M HEDTLER | 71 MICHAEL LN | whitinsvilue | MA | 01588 | 71 MICHAEL LN |
| 7 | 364 |  | kelley thomas a | Nicole f keliey | 211 MASON RD | whitinsvilie | MA | 01588 | 211 MASON RD |
| 7 | 365 |  | Jorritsma RICHARD L | RIA H JORRITSMA, TE | 223 MASON RD | whitinsvilue | MA | 01588 | 223 MASON RD |
| 7 | 366 |  | TUCKER BRANDON P | c/o michaed joseph lang | 60 CANTON ST | Sharon | MA | 02067 | 40 ACORN RD |
| 7 | 367 |  | white matthew J | kelly a white | 34 ACORN RD | whitinsvilile | MA | 01588 | 34 ACORN RD |
| 7 | 368 |  | broors amy l |  | 28 ACORN RD | whitinsville | MA | 01588 | 28 ACORN RD |
| 7 | 369 |  | Stefaniak michael J JR te | anne b Stefaniak | 22 ACORN RD | whitinsvilie | MA | 01588 | 22 ACORN RD |
| 7 | 373 |  | perry steven m | kathleen b perry | 25 ACORn RD | whitinsville | MA | 01588 | 25 ACORN RD |
| 7 | 374 |  | Coe james t |  | 29 ACORN RD | whitinsvilue | MA | 01588 | 29 ACORN RD |
| 7 | 375 |  | duffy susan b | C/O JOShua \& SARAH RODHE | 35 ACORN RD | whitinsvilide | MA | 01588 | 35 ACORN RD |
| 7 | 376 |  | LESSARD VICTOR I | FRANCES M Lessard | 251 MASON RD | whitinsvilie | MA | 01588 | 251 MASON RD |
| 7 | 377 |  | EbBeling ronald J | C/o Steven \& brianne susel | 263 MASON RD | whitinsvilue | MA | 01588 | 263 MASON RD |
| 7 | 378 |  | GARD GERALD I | JEAN M GARD | 277 MASON RD | whitinsvilue | MA | 01588 | 277 MASON RD |
| 7 | 379 |  | SWEetman robert d | Joann sweetman | 291 MASON RD | whitinsvilue | MA | 01588 | 291 MASON RD |
| 7 | 380 |  | edwards Michael a | MARGARET K edwards | 308 SWIFT RD | whitinsvilie | MA | 01588 | 308 SWIFT RD |
| 7 | 381 |  | bliss burt j | SHERYI L BLISS | 298 SWIET RD | whitinsvilie | MA | 01588 | 298 SWIFT RD |
| 7 | 382 |  | gagnon david r | EdNa I GAGNon, TE | 286 SWIET RD | whitinsvilie | MA | 01588 | 286 SWIFT RD |
| 7 | 386 |  | Arbuckle priscilla s | john D Arbuckle | 82 Fairlamin St | whitinsvilie | MA | 01588 | 82 FAIRLAWN ST |
| 7 | 387 |  | MORRISSETTE PATRICIA F |  | 94 SULLIVAN DR | whitinsvilie | MA | 01588 | 94 SULLIVAN DR |
| 6A | 2 |  | bailey stella C, i.e. | C/O THOMAS \& Christine scanlon | 236 No MAIN ST | whitinsvilie | MA | 01588 | 236 NO MAIN ST |
| 6 A | 3 |  | GONYNOR ROBERT |  | 222 NO MAIN ST | whitinsville | MA | 01588 | 222 NO MAIN ST |
| 6A | 4 |  | tran steve |  | 2077 WISTERIA LN | middleburg | FL | 32068-5037 | 206 NO MAIN ST |
| 6A | 7 |  | ROONEY LAWRENCE | CHERRY H ROONEY | 25 WEST HILI RD | mendon | MA | 01756 | 34-44 OVERLOOK ST |
| 6 A | 8 |  | CC\&L Properties, llc | george \& Laura pappas | 4 budreau ave | mililbury | MA | 01527 | 22-32 OVERLOOK ST |
| 6A | 11 |  | LORD WILliam J | DANIEL E LORD | 1-3 OVERLOOK ST | whitinsville | MA | 01588 | 1-3 OVERLOOK ST |
| 6A | 14 |  | SOUTH MIDDLESEX NON-PROFIT | housing corporation | 7 BISHOP ST | framingham | MA | 01702 | 21-31 OVERLOOK ST |
| 6 A | 15 |  | SOUTH MIDDLESEX NON-PROFIT | HOUSING CORPORATION | 7 bishof St | Framingham | MA | 01702 | 33-43 OVERLOOK ST |
| 6A | 16 |  | CARROLL DAVID JR | michelle a carroll | PO BOX 333 | whitinsvilile | MA | 01588 | 182-184 NO MAIN ST |
| 6A | 19 |  | barer gregory | heather baker, te | 1.50 NO MAIN ST | whitinsvilile | MA | 01588 | 150 NO MAIN ST |
| 6 A | 21 |  | humphrey branden J | Sharon R humphrey, te | PO BOX 467 | goffstown | NH | 03045 | 108-112 NO MAIN ST |
| 6A | 22 |  | Mello paul j, sR | donna melio, te | 1-3 Crescent st | whitinsville | MA | 01588 | 1-3 Crescent st |
| 6A | 23 |  | kent ronald r | DAVIde e tremblay, te | 5-7 Crescent st | whitinsville | MA | 01588 | 5-7 Crescent st |
| 6 A | 24 |  | MAYER Christopher j | julie laplante | 18-22 CRESCENT ST | whitinsvilue | MA | 01588 | 18-22 CRESCENT ST |
| 6A | 25 |  | haggerty richard R | C/O michael ramd \& kelly royce | 14-16 CRescent st | whitinsvilie | MA | 01588 | 14-16 CRESCENT ST |
| 6 A | 26 |  | guiou diane |  | 10-12 CRESCENT ST | whitinsvilie | MA | 01588 | 10-12 CRESCENT ST |
| 6A | 27 |  | kAmishlian nicole | C/O Steven lloyd dearborn | 6-8 CRESCENT ST | whitinsville | MA | 01588 | 6-8 CRESCENT ST |
| 6 A | 28 |  | tharsille, Llc |  | P O box 341 | manchaug | MA | 01526 | 2-4 Arcade st |
| 6 A | 36 |  | beaudotn harriet | MICHAEL BONET \& NATASHA SANTORO,TC | 5 ARCADE ST | whitinsvilie | MA | 01588 | 5-7 ARCADE ST |
| 6 A | 37 |  | PLANT BRIAN | Colleen m Plant, te | 1 ARCADE ST | whitinsvilile | MA | 01588 | 1-3 Arcade st |
| 6A | 38 |  | mCLAUGHLIN NANCY A | COLLeen m mclaughlin | 4 Crescent street | whitinsvilue | MA | 01588 | 2-4 CRESCENT ST |
| 6A | 39 |  | oikle arnold l | Carol lee oikle | 329 Hazel St | UXBRIDGE | MA | 01569 | 96-98 NO MAIN ST |
| 6 A | 40 |  | White ronald l | kathleen a white | 88 NO MAIN ST | whitinsvilie | MA | 01588 | 86 No MAIN ST |
| 6 A | 123 |  | drosidis konstantinos | Eleni drosidis | 199 NO MAIN ST | whitinsvilie | MA | 01588 | 205 NO MAIN ST |
| 4/11/2019 4:22:01PM |  |  |  |  |  |  |  |  |  |

## ABBUTTERS LISTING

## NORTHBRIDGE, MA

| Map | Block | Lot | Unit | Owner ${ }^{\text {c }}$ S Name | Co Owner~s Name | Address | City | ST | zip | Parcel Location |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6A | 124 |  |  | TINKLENBERG JACOB K | BEVERLY R TINKIENBERG, TE | 225 NO MAIN ST | whitinsvilue | MA | 01588 | NO MAIN ST |
| 6A | 125 |  |  | tinklenberg jacob | beverly tinklenberg | 225 NO MAIN ST | whitinsville | MA | 01558 | 225 No MAIN ST |
| 6A | 126 |  |  | nye Steven R |  | 233 NO MAIN ST | whitinsville | MA | 01588 | 233 NO MAIN ST |
| 6A | 127 |  |  | CRAY Brian R | CRAY JUlie a | 241 NO MAIN ST | whitinsville | MA | 01588 | 241 No MAIN ST |
| 6A | 128 |  |  | Whitinsville redevelopment tr | Sidney covich trustee | 1 main Street | whitinsville | MA | 01588 | No MAIN ST |
| 6A | 149 |  |  | REINHOLT Ashley J | Johnathon william reinholt te | 18 bunkerhili pkwy | WEST BOYLSton | MA | 01583-2004 | 11 OVERLOOR ST |
| 6 A | 150 |  |  | deutsche bank nat trust co | C/O IRISH GREGOR | 546 FOWIER RD | NORTHBRIDGE | MA | 01534 | 13 OVERLOOK ST |
| 6 A | 151 |  |  | faicione robert J |  | 15 OVERLOOK ST | whitinsvilile | MA | 01588 | 15 OVERLOOK ST |
| 6A | 152 |  |  | LSE9 MASTER PARTICIPATION TRUST | us bank trust, nA, trustee | C/0 \% RESICAP | AtLANTA | GA | 30326 | 17 OVERLOOR ST |
| 6A | 153 |  |  | ESCOTt DOnNa J |  | 19 OVERLOOK ST | whitinsville | MA | 01588 | 19 OVERLOOR ST |
| 6A | 161 |  |  | CRUZ, VICTOR RAEAEL | c/o vs Cruz realty lic | 30 KINGSton ST | LAWRENCE | MA | 01843 | 60 OVERLOOK ST |
| 6A | 162 |  |  | CRUZ VICtor | c/o vs cruz realty lic | 30 KINGSton St | LAWRENCE | MA | 01843 | 62 OVERLOOK ST |
| 6A | 163 |  |  | CRUZ VICtor | c/o vs Cruz realty lic | 30 KINGSTON ST | Lamrence | MA | 01843 | 64 OVERLOOK ST |
| 6A | 164 |  |  | the brady impact | c/o vs cruz realty lic | 30 kINGSton St | LAWRENCE | MA | 01843 | 66 OVERLOOK ST |
| 6A | 165 |  |  | eldridge linda | WESLEY ELDRIDGE, TE | 70 BIRCH ST APT 3 | WORCESter | MA | 01603-2726 | 68 OVERLOOK ST |
| 6 A | 178 |  |  | cruz victor | c/o vs cruz reaity lic | 30 KINGSton ST | LAWRENCE | MA | 01843 | 58 OVERLOOK ST |
| 6A | 218 |  |  | Billmyer michael | JAnet billmyer, te | 5 OVERLOOK ST | whitinsvilie | MA | 01588 | 5 OVERLOOK ST |
| 6A | 219 |  |  | haden kyle a | C/O TRISHA/DANIEL BEGNOCHE | 7 OVErLOoK ST | whitinsvilie | MA | 01588 | 7 OVERLOOK ST |
| 6A | 276 |  |  | GADOURY HOMES LLC |  | 6 Reservoir ave | manchaug | MA | 01526 | 46 OVERLOOK ST |
| 6A | 277 |  |  | 46-56 OVErlook st condominium | C/O GAdoury homes lic | P O BOX 495 | manchaug | MA | 01526 | 48 OVERLOOK ST |
| 6 A | 278 |  |  | 46-56 OVERLOOK ST Condominium | c/o gadoury homes llc | P O box 495 | manchaug | MA | 01526 | 50 OVERLOOK ST |
| 6A | 279 |  |  | 46-56 OVERLOOK ST CONDOMINIUM | C/O GADOURY HOMES LLC | P O BOX 495 | manchaug | MA | 01526 | 52 OVERLOOK ST |
| 6 A | 280 |  |  | 46-56 OVERLOOK ST CONDOMINIUM | C/O GAdoury homes luc | P O BOX 495 | MANCHAUG | MA | 01526 | 54 OVERLOOK ST |
| 6A | 281 |  |  | 46-56 OVERLOOK ST Condominium | C/O Gadoury homes llc | PO BOX 495 | manchaug | MA | 01526 | 56 OVERLOOK ST |

CHILTON KENDELL A
125 BROOKWAY DR
NORTHBRIDGE, MA 01534

GUO BINGZHU
125 BROOKWAY DR
NORTHBRIDGE, MA 01534

YOUNGSMA ALVIN H TR
MARY L YOUNGSMA TR
269 NO MAIN ST
WHITINSVILLE, MA 01588

MONTECALVO JOSEPH J
MARGARET B MONTECALVO
279 NO MAIN ST
WHITINSVILLE, MA 01588

TOWN OF NORTHBRIDGE
N/A
NORTHBRIDGE, MA 01534

K T K M REALTY TRUST
611 LINCOLN ST
FRANKLIN, MA 02038

CONNOLLY JOHN C
KAREN J CONNOLLY
97 TRACEY DRIVE
WHITINSVILLE, MA 01588

O'DONNELL GLENN E
DONNA O'DONNELL
89 MASON RD
WHITINSVILLE, MA 01588

LAYDON JOSEPH T CHRISTINA P LAYDON
63 MASON RD
WHITINSVILLE, MA 01588

CIOFFI ALFRED
CYNTHIA CIOFFI
82 MASON RD
WHITINSVILLE, MA 01588

KELLY SUSAN A
SEAN J KELLY
70 MASON RD
WHITINSVILLE, MA 01588

PIXLEY GERALD W
SUSAN M PIXLEY, TE
56 MASON RD
WHITINSVILLE, MA 01588

HEFFERNAN TIMOTHY M JACQUELYN M LYONS-HEFFERNAN 48 MASON RD
WHITINSVILLE, MA 01588

NAU LURANA M
66 FAIRLAWN ST
WHITINSVILLE, MA 01588

DAWSON MICHAEL J
C/O TOBIAS M CONIO
60 FAIRLAWN ST
WHITINSVILLE, MA 01588

BAXENDALE JAMES F TRUSTEE
BAXENDALE REALTY TRUST
52 FAIRLAWN ST
WHITINSVILLE, MA 01588

THIBODEAU RITA P, LE
M STOCKHAUS, L SOHIGIAN, S KURAS, T 34 FAIRLAWN ST
WHITINSVILLE, MA 01588

MARSHALL BRIAN
LYNN MARSHALL
26 FAIRLAWN ST
WHITINSVILLE, MA 01588

WHITAKER CHRISTINA A 20 FAIRLAWN ST
WHITINSVILLE, MA 01588

CAMPBELL JESSICA L
JONATHON S CAMPBELL TE 352 NO MAIN ST
WHITINSVILLE, MA 01588

TAYLOR JOSEPH R
C/O JOSEPH R TAYLOR TRUSTEE
344 NO MAIN ST
WHITINSVILLE, MA 01588

KUINDERSMA MARK
DIANE M BEAULIEU, JT
70 FAIRLAWN ST
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POULIOT JEANNETTE L BRAIN G POULIOT, TC 330-332 NO MAIN ST WHITINSVILLE, MA 01588

BEDIGIAN JAMES D
320 NO MAIN ST
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SHANNAHAN JOHN P
C/O ANDREW \& ABAGAIL YANCO 306 NO MAIN ST
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WIERSMA BERNARD TRUSTEE
B \& M WIERSMA LIVING TRUST 104 SULLIVAN DR
WHITINSVILLE, MA 01588

BOWMAN RONALD
NANCY BOWMAN
82 SULLIVAN DR
WHITINSVILLE, MA 01588

GARRITY ROBERT M
74 SULLIVAN DR
WHITINSVILLE, MA 01588

DESPLECHIIN DAWN M
WILLIAM D ISON, JT 64 SULLIVAN DR WHITINSVILLE, MA 01588

GILE CARROLL G
C/O JOCELYN L ARN
56 SULLIVAN DR
WHITINSVILLE, MA 01588

DOBELBOWER JAKE
ASHLEY L DOBELBOWER
48 SULLIVAN DR
WHITINSVILLE, MA 01588

BROWN MICHAEL L
38 SULLIVAN DR
WHITINSVILLE, MA 01588

POWERS EILEEN
NANCY A \& THOMAS P POWERS, JT
25 SULLIVAN DR
WHITINSVILLE, MA 01588

POWERS EILEEN
NANCY A \& THOMAS P POWERS, JT 25 SULLIVAN DR WHITINSVILLE, MA 01588

FREMEAU MARK J
LYNNE VALLEY FREMEAU
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C/O MEGHAN M WINCHELL \& SUSAN BU] 286 NO MAIN ST
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C/O JASON BALL \& DONNA EVANS
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HOLLY L AUSTIN TE
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CRAIG FRANCINE
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ZANELLA PATRICK T<br>TARA A ZANELLA<br>254 MASON RD<br>WHITINSVILLE, MA 01588

VALIS DAVID O
LINDA M VALIS
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BARIS CHARLES R
CATHERINE F BARIS
230 MASON RD
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COURTEMANCHE JOHN
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BROOKS JOHN LEROY
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STOCKWELL EDWARD R SR MARGARET B STOCKWELL, TE 168 MASON RD WHITINSVILLE, MA 01588

KELLEHER SEAN D
C/O JEREMY HARRIS/LESLIE R COSGRO 156 MASON RD
WHITINSVILLE, MA 01588

KOUREY NICHOLAS W C/O SCOTT \& SAMANTHA MURDOUGH 146 MASON RD
WHITINSVILLE, MA 01588

GUGLIELMO KENNETH R
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GAMBON THOMAS M
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SULLIVAN BRIAN J 68 EVERGREEN CR WHITINSVILLE, MA 01588

FORTIN LIVING TRUST
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HAY CRAIG D
MARY E HAY,TE
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CAHALANE JONATHAN V
DENISE E CAHALANE 20 EVERGREEN CR WHITINSVILLE, MA 01588

NEWELL KENNETH S BRENDA L NEWELL 19 EVERGREEN CR WHITINSVILLE, MA 01588

HAWKES CHARLES B KRISTINE B HAWKES 126 FAIRLAWN ST WHITINSVILLE, MA 01588

FROMENT KRISTINE A DAVID MA FROMENT,TE 31 EVERGREEN CR WHITINSVILLE, MA 01588

MALONE MICHAEL P MELISSA A MALONE 41 EVERGREEN CR WHITINSVILLE, MA 01588

COGLIANDRO PAUL D
SUSAN M. COGLIANDRO
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PILEGGI MARK \& DAVID PILEGGI JR. TRT C/O PILEGGI IRREVOCABLE TRUST 65 EVERGREEN CR WHITINSVILLE, MA 01588

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PILEGGI DAVID J JR
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DEMBROWSKI STEPHEN J MARIE A DEMBROWSKI 103 MASON RD
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GAY BRUCE C
MARGARET M GAY
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DER MUGRDITCHIAN MARK
CYNTHIA DER MUGRDITCHIAN
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ROSSELLI ANTHONY J
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ROBINSON DANIEL P
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BOL NICHOLAS P
KELLY S BOL, TE
69 KERRY LN
WHITINSVILLE, MA 01588

DURGIN WILLIAM R
LINDA F DURGIN
81 KERRY LANE
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OUILLETTE DAVID J
MARYANN OUILLETTE
93 KERRY LN
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BARKLEY JOHN C
BETH A BARKLEY
175 MASON RD
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BANNING ROBERT A
ELIZABETH A BANNING
191 MASON RD
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COOK BRIAN D
KATE E COOK,TE
76 MICHAEL LN WHITINSVILLE, MA 01588

HENDERSON CHRISTOPHER
KAREN D HENDERSON
64 MICHAEL LN
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TOWNSEND DAVID J
JESSICA M TOWNSEND
50 MICHAEL LN
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CALUORI MICHAEL JR
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STEFANIAK MICHAEL J JR TE
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PERRY STEVEN M KATHLEEN B PERRY
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COE JAMES T
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DUFFY SUSAN B
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GARD GERALD I
JEAN M GARD
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SWEETMAN ROBERT D
JOANN SWEETMAN
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EDWARDS MICHAEL A
MARGARET K EDWARDS
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SHERYL L BLISS
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GAGNON DAVID R
EDNA I GAGNON, TE
286 SWIFT RD
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ARBUCKLE PRISCILLA S
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MORRISSETTE PATRICIA F
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BAILEY STELLA C, L.E.
C/O THOMAS \& CHRISTINE SCANLON 236 NO MAIN ST
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DONNA MELLO, TE
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5-7 CRESCENT ST
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MAYER CHRISTOPHER J
JULIE LAPLANTE 18-22 CRESCENT ST
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HAGGERTY RICHARD R
C/O MICHAEL RAAD \& KELLY ROYCE
14-16 CRESCENT ST
WHITINSVILLE, MA 01588

GUIOU DIANE
10-12 CRESCENT ST
WHITINSVILLE, MA 01588

KAMISHLIAN NICOLE
C/O STEVEN LLOYD DEARBORN
6-8 CRESCENT ST
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THARSILLE, LLC
P O BOX 341
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BEAUDOIN HARRIET
MICHAEL BONET \& NATASHA SANTORO.
5 ARCADE ST
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PLANT BRIAN
COLLEEN M PLANT, TE
1 ARCADE ST
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MCLAUGHLIN NANCY A
COLLEEN M MCLAUGHLIN
4 CRESCENT STREET
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OIKLE ARNOLD L
CAROL LEE OIKLE
329 HAZEL ST
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WHITE RONALD L
KATHLEEN A WHITE
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DROSIDIS KONSTANTINOS
ELENI DROSIDIS
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WHITINSVILLE, MA 01588

TINKLENBERG JACOB K
BEVERLY R TINKLENBERG, TE
225 NO MAIN ST
WHITINSVILLE, MA 01588

TINKLENBERG JACOB
BEVERLY TINKLENBERG
225 NO MAIN ST
WHITINSVILLE, MA 01558

NYE STEVEN R
233 NO MAIN ST
WHITINSVILLE, MA 01588

CRAY BRIAN R
CRAY JULIE A
241 NO MAIN ST
WHITINSVILLE, MA 01588

WHITINSVILLE REDEVELOPMENT TR
SIDNEY COVICH TRUSTEE
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WHITINSVILLE, MA 01588

REINHOLT ASHLEY J
JOHNATHON WILLIAM REINHOLT TE
18 BUNKERHILL PKWY
WEST BOYLSTON, MA 01583-2004

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C/O IRISH GREGOR
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US BANK TRUST, NA, TRUSTEE
C/O \% RESICAP
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CRUZ, VICTOR RAFAEL C/O VS CRUZ REALTY LLC
30 KINGSTON ST
LAWRENCE, MA 01843

CRUZ VICTOR
C/O VS CRUZ REALTY LLC
30 KINGSTON ST
LAWRENCE, MA 01843

CRUZ VICTOR
C/O VS CRUZ REALTY LLC 30 KINGSTON ST
LAWRENCE, MA 01843

THE BRADY IMPACT
C/O VS CRUZ REALTY LLC 30 KINGSTON ST
LAWRENCE, MA 01843

ELDRIDGE LINDA
WESLEY ELDRIDGE, TE
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WORCESTER, MA 01603-2726

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C/O VS CRUZ REALTY LLC 30 KINGSTON ST
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HADEN KYLE A
C/O TRISHA/DANIEL BEGNOCHE
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WHITINSVILLE, MA 01588

GADOURY HOMES LLC
6 RESERVOIR AVE
MANCHAUG, MA 01526

46-56 OVERLOOK ST CONDOMINIUM
C/O GADOURY HOMES LLC
P O BOX 495
MANCHAUG, MA 01526

46-56 OVERLOOK ST CONDOMINIUM
C/O GADOURY HOMES LLC
P O BOX 495
MANCHAUG, MA 01526

46-56 OVERLOOK ST CONDOMINIUM
C/O GADOURY HOMES LLC
P O BOX 495
9 RESERVOIR AVE
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46-56 OVERLOOK ST CONDOMINIUM
C/O GADOURY HOMES LLC
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February 28, 2019
Mr. James Sheehan, Building Inspector
Town of Northbridge
Aldrich School Town Hall Annex
14 Hill Street
Whitinsville, MA 01588

## RE: W. Edward Balmer Elementary School - Zoning Bylaws Analysis

Dear Jim,
Following is our analysis of the Northbridge Zoning Bylaws as they apply to the project to construct a new Grades PK-5 elementary school on the site of the existing Balmer school, which will also involve the Vail Field parcel as part of the project. As requested, we are showing where the project meets the requirements of the bylaws, where it does not, and the mitigating factors that will demonstrate in our professional opinion, that there will be no substantial detriment to the public good or undue burdens placed on the town if it allows the non-conforming aspects of the project to be approved by waiver or variance. This letter is not an exhaustive analysis; only portions of the Zoning Bylaw that have direct bearing on the proposed development are included here.
I. LAND USE, VAIL FIELD

The Town Legal Counsel, KP Law, through its deed research, has determined that Vail Field is not subject to Article 97 (Change of Use of Public Parklands) regulations (letter attached). Furthermore, all existing athletic facilities are proposed to be replaced in-kind, in a new configuration, as part of the proposed site plan.
II. ZONING BYLAWS ANALYSIS

173-4 ZONING MAP:
The project site sits partially in two zones. The south portion (Crescent Street frontage) including Vail Field and some portion of the school parcel sits in zone R-5. The rear portion which includes the balance of the school parcel sits in zone R-2. The majority of the new school is located in the R-2 zone, which is used below for side yard setback calculations. The site is not part of any Overlay District, and is not located in a Floodway or Flood Plain district.


Figure 1 - Northbridge Zoning Map (partial) - May 2016, with property identified

## ARCHITECTS

## PROJECT MANAGERS

[^0]173-12 USE REGULATIONS:
Community Public Educational Facilities are a permitted use in Zones R-2 and R-5. (Table 173-12, Att. 2)
173-13.2 EROSION CONTROL:
The project will be subject to MA law and guidelines for construction erosion control, and an Erosion Control Plan will be submitted to the Town as part of the construction permit process. (Table 173-18.2. C and D)

173-20 HEIGHT AND BULK REGULATIONS:

TABLE 1: Dimensional Requirements per Zoning Bylaws (173-20 + 173 - Att. 1)

|  | Min. Lot Area (sq. ft.) | Min. <br> Contiguous Frontage | Min. Front Yard Setback | Min. Side Yard Setback | Min. Rear Yard Setback | Max. Height in Stories | Max. Height in Feet* | Max. Total Lot Coverage (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Required R-2 | 20,000 | 100' | 40' | 10' | 40' | 2.5 | $35^{\prime}$ | 20\% |
| Required R-5 | 5,000 | $60^{\prime}$ | 15 | 8' | 20' | 3 | 45 | 50\% |
| Existing** | 1,310,285 | 730' | $30^{\prime}$ | $50^{\prime}$ | 310' | 2 | $23^{\prime}-6{ }^{\prime \prime \prime}$ | 4 \% |
| Proposed New Project Actual Measurements (Re. R-2 zone) | 1,310,285 | 730' | 565.64' | 384.7' west 42.65' east* | 307.15 | 3* | 44'-4" * | 5.65 \% |

* "Any maximum height permitted shall not apply to a community facility provided that the side and rear yards or setbacks required in the district for the highest permitted principal structure shall be increased two feet in width for each foot by which the height of such structure exceeds the height permitted in the district." See calculation below.
** Existing calculations are based on property ID: 7-138 (parcel the school building sits within.)


## 173-20 SIDE YARD SETBACK CALCULATION:

Exception for Community Facilities (Sec 173-20: Table Notes)
Height $43^{\prime}-10^{\prime \prime}$ to cornice; nominally $44^{\prime}-4^{\prime \prime}$ to average grade.

| R-2 Allowable Height $=$ | $35^{\prime}$ |
| :--- | :--- |
| Proposed Height $=44^{\prime}-4^{\prime \prime}$ | $\left(44.33^{\prime}\right)$ |
| Height Delta $=$ | $9.33^{\prime}$ |
| Setback multiplier $=$ | 2.0 |
| Added Setback | $18.66^{\prime}$ |
| Base Side Setback | $10^{\prime}$ |
| Required Side Setback | $\mathbf{2 8 . 6 6}$ |

## Actual Side yard Setback <br> 42.65' at northeast corner

173-27 OFF-STREET PARKING AND LOADING REQUIREMENTS:
For reference, the existing structure has 96 paved, striped, legitimate parking spaces, and two loading spaces adjacent to the loading dock.

Parking:
Zoning Requirement: Community Facilities - Schools: 1 space per 300 NSF (table in Sec 173-27.C)
Building NSF $=111,568$ NSF
Zoning Requires 372 parking spaces
Desired Parking Program per District Working Group:

| 156 Staff + 24 Visitors | 180 spaces |
| :--- | :--- |
| Additional Event Parking | 89 spaces |
| Total Parking on Site Plan | 246 spaces |
| Variance or Waiver for | $\mathbf{1 2 6}$ spaces |

We are submitting an "Overflow Parking Plan" that will yield an additional 54 spaces (drawing attached). This brings the total on-site parking capacity to 300 spaces.

Loading Areas:
Zoning requires 1 per 7,500 NSF + 1 per 15,000 NSF in excess (table 2 in Sec.173-27.C)
Building NSF = 111,568 NSF
Zoning requires: 8 loading spaces
Project has: 2 loading spaces
Seeking Variance or Waiver for 6 loading spaces
Per the request of the Technical Review Committee at our $1 / 23 / 19$ meeting, we are submitting a verification of the school's parking needs as well as a Parking and Event Analysis which shows that there are no likely scenarios that will exceed the total onsite parking capacity. Most scenarios will easily be accommodated with the proposed 246 spaces, and the few high-capacity events will be accommodated using the Overflow plan for 300 spaces. (Documents attached)

Additional Zoning Requirements:
Proposed Parking and Loading Spaces are all on the same lot as the building served. (Sec.173-27.D.1, .2)
Proposed spaces are $9^{\prime} \times 18^{\prime}$ with $24^{\prime}$ drive aisle in lot configurations. Parallel parking spaces in the Overflow Plan are 9' $\mathrm{x} 22^{\prime}$ with a minimum $12^{\prime}$ drive lane accessing them. (Sec.173-27.D.3)

The proposed number of driveways accessing the public way (Crescent Street) is limited to two. (Sec.173-27.D.4)
Proposed two-way drive ways are 22 feet wide, two lanes of 11 feet. (Sec.173-27.D.5)
Loading spaces shall be 600 SF for the first 7,500 NSF and 500 SF for each additional 15,000 NSF. There are two spaces of 600 SF . The project has two proposed loading spaces of 900 SF that will accommodate a semi-trailer or straight truck. (Sec.173-27.D.9)

Handicapped parking spaces are provided in accordance with MAAB and ADA requirements. There are $8 \mathrm{H} / \mathrm{C}$ spaces on the site, where a minimum of 7 are required. (Sec.173-27.D.12; MAAB 521 CMR 23.2.1)

The balance of regulations 173-27.D 1-13 have been incorporated in the site plans.
The proposed plan includes landscaping plant materials (primarily trees to screen and shade the parking lot areas. (173-27.F.3-(a)-(c) )

173-28 AREA, CONSTRUCTION AND LIGHTING STANDARDS
The west parking lot is approximately 100 feet and 20-30 feet down-slope from neighbors to the west. Parking lot islands feature trees which will screen the parking from views from above. It is our interpretation that solid screen walls are not required in this condition. The east parking is screened by both solid 6 ' stockade fencing at the property line, and dense evergreen shrubbery between the fence and the parking lots. Other provisions of this section are being complied with ( D - lighting) or are not applicable (B, C). (173-28.A-D)

Please contact me if you have any question on the above material, and we look forward to continuing the permitting process for this project.

Sincerely,
DORE \& WHITTIER ARCHITECTS, INC.
Architects • Project Managers


Tom Hengelsberg, AIA
Project Manager
Attachments
cc: File

101 Arch Street, Boston, MA 02110 Tel: 617.556.0007 |Fax: 617.654.1735

Northbridge School Building Committee
Town Hall
7 Main Street
Whitinsville, MA 01588
Re: W. Edward Balmer Elementary School, Executive Office of Energy and Environmental Affairs Article 97 Land Disposition Policy

Dear Members of the School Building Committee:
I have reviewed the identified deed for the Balmer School site - deed of Whitin Machine Works to Town of Northbridge dated April 24, 1963 and recorded with the Worcester Registry of Deeds in Book 4369, Page 342. The deed conveyed 4 parcels to the Town. Parcel 1 is land on the northwesterly side of Crescent Street and the northeasterly side of North Main Street, said to contain 9.04 acres and Parcel 2 is a parcel northwesterly of Parcel 1 said to contain 21.04 acres. The copy of the deed provided by the Assessors' office includes the annotation that the land conveyed encompasses Assessors' Map 7, parcels 138 and 141. According to the Assessors' property card record for the Balmer School property, the school site has an address of 11 Crescent Street, is shown as parcel 138 on Assessors' Map 7, and contains 30.04 acres. (Assessors' Map 7 shows parcel 138 as containing 21.04 acres, with the designation "Balmer School" and parcel 141 as containing 9.04 acres.) Accordingly, it is my understanding that the school site is Parcel 1 and Parcel 2 described in the deed. (Parcel 3 is described as land on the northerly side of Plummer Road a/k/a Church Street, between Providence Road and Quaker Street, consisting of 2.51 acres; and Parcel 4 is described as land on the westerly side of Linwood Avenue, consisting of 30,014 square feet.)

The deed to the Balmer School site includes no statement of use limitations or restriction on Town use of the land. Therefore, it is my opinion that the deed does not impose a limitation that would make the site subject to Article 97 of the Amendments to the Massachusetts Constitution, which includes a prohibition against the sale or change in use of public parkland without special approval by a two-thirds roll call vote of the Legislature.

Article 97 can apply when land acquired without any use restriction is subsequently subjected to a restriction by a document recorded with the Registry of Deeds. See Smith v. City of Westfield, 90 Mass. App. Ct. 80, 82 (2016). It is my understanding that the Town is not aware of any such recorded restriction or similar action for the Balmer School site. My on-line search of Worcester Registry of Deeds records, by street - Crescent Street, did not reveal any subsequent recorded restriction.

## KP LAW

Northbridge School Building Committee
August 31, 2017
Page 2
You have also informed me that a portion of the Balmer School site contains a recreational field area, known as Vail Field. In that regard, I reviewed certain votes taken at the March 12, 1963 Annual Town Meeting regarding the Town's acceptance of land from Whitin Machine Works - one parcel of approximately 6.22 acres "known as Vail Field . . . to be used for recreational purposes only" (Article 13) and one parcel of approximately 23.25 acres "adjacent to Vail Field . . . to be used as a school site only" (Article 16). Although the stated acreage for these two parcels is different from the parcel sizes reflected in the deed referenced above and the parcel sizes being carried on the Northbridge Assessors' records, it is my understanding that the votes refer to the parcels conveyed by that deed. The Vail Field designation for the smaller parcel appears to pre-exist any transfer to the Town from Whitin Machine Works. In any event, though, creation of a restriction for purposes of Article 97 of the Amendments to the Massachusetts Constitution requires an instrument recorded at the Registry of Deeds. See Mahajan v. Department. of Environmental Protection, 464 Mass. 604, 615 - 616 (2013), citing Selectmen of Hanson v. Lindsay, 444 Mass. 502 (2005). No such instrument has been identified. Accordingly, the existence of these votes, with no restrictive instrument recorded at the Registry of Deeds, does not alter the opinion that the Balmer School site is not subject to Article 97.

In accordance with the foregoing, and in response to your further question of August 28, 2017, it is my view that the so-called Vail Field portion of the site may be used for non-recreational purposes and that the other portions of the site may be used for recreational purposes.

Please contact me if you have any further questions on this matter.


DJD/man
cc: Board of Selectmen

589268 v.2/NBRI/0001


W.E. BALMER ELEMENTARY SCHOOL

DORE \& WHITTIER ARCHITECTS
DESIGN DEVELOPMENT ZONING SUBMISSION - PARKING ANALYSIS
TABLE 1 - STAFF COUNT
Verified with School Administration 1/31/19

| SPACE | QUAN | $\begin{array}{\|c} \hline \text { ADULTS } \\ \text { BASED } \\ \text { IN } \\ \text { EACH } \\ \hline \end{array}$ | FTE | STUDENTS IN EACH ${ }^{1}$ | TOTAL STUDENTS | Remarks |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PK CRS | 4 | 1 | 4 | 18 | 72 |  |
| PK-K SPED | 1 | 1 | 1 | 12 | 12 |  |
| K CRS | 9 | 1 | 9 | 18 | 162 |  |
| GRADE 1-5 CRS | 40 | 1 | 40 | 23 | 920 |  |
| 1-2 SPED | 2 | 3 | 6 | 12 | 24 |  |
| 3-5 SPED | 2 | 3 | 6 | 12 | 24 |  |
| RESOURCE ROOM | 3 | 1 | 3 |  |  | STUDENTS COUNTED ABOVE |
| STUDENT SERVICES | 2 | 26 | 52 |  |  | PROFESSIONALS WORK IN CLASSROOMS ABOVE |
| ART | 2 | 1 | 2 |  |  | STUDENTS COUNTED ABOVE |
| MUSIC | 2 | 1 | 2 |  |  | STUDENTS COUNTED ABOVE |
| GYMNASIUM | 1 | 2 | 2 |  |  | STUDENTS COUNTED ABOVE |
| LIBRARY | 1 | 2 | 2 |  |  | STUDENTS COUNTED ABOVE |
| MAKER | 1 | 1 | 1 |  |  | STUDENTS COUNTED ABOVE |
| OT/PT | 1 | 2 | 2 |  |  | STUDENTS COUNTED ABOVE |
| ADMIN + NURSE |  |  | 16 |  |  | INCL PRINCIPAL OFFICES ON LEVEL 2+3 |
| TITLE 1 OFFICE |  |  | 1 |  |  |  |
| KITCHEN |  |  | 5 |  |  |  |
| MAINTENANCE STAFF |  |  | 2 |  |  |  |
| SUBTOTAL - FTE |  |  | 156 |  | 1214 |  |
|  |  |  |  |  |  |  |
| VISITORS |  |  |  |  |  |  |
| ITINERANT PROFESSIONALS |  |  | 2 |  |  | Not full time - in building for no more than 2 hours |
| VOLUNTEERS |  |  | 4 |  |  | Sporadic, usually present for most of the school day |
| VISITORS |  |  | 18 |  |  | 3 meetings a day $\times 6$ people, could be concurrent |
| SUBTOTAL |  |  | 24 |  |  |  |
|  |  |  |  |  |  |  |
| TOTALS |  |  | 180 |  | 1214 |  |

${ }^{1}$ Reflects maximum enrollment, not actual present enrollment.

## W.E. BALMER ELEMENTARY SCHOOL

## DORE \& WHITTIER ARCHITECTS

DESIGN DEVELOPMENT ZONING SUBMISSION - PARKING ANALYSIS

## TABLE 2 - PARKING AND EVENT ANALYSIS

Proposed Parking Spaces $246+$ Overflow Spaces $54=300$ Total Spaces Onsite Maximum
Table shows the maximum number of cars parked for any given time period/ scenario. Cells highlighted yellow indicate scenario totals above the number of conventional spaces. None of the scenarios exceed the total onsite maximum number of parking spaces, including overflow spaces

| TIME OF DAY | EVENT/ CONDITION | FREQUENCY | PARKING (LONG TERM) | PARKING (S/T VISITOR $<2$ hours) | $\begin{array}{\|l} \hline \text { QUEUE } \\ \text { SPACE } \end{array}$ | LOADING <br> SPACE <br> (Semi <br> Truck) | REMARKS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SCHOOL DAY |  |  |  |  |  |  |  |
| 6:00 AM - 7:45 AM | Supply Deliveries | Daily M-F |  |  |  | 2 | Various deliveries throughout week, rarely more than one truck at a time. |
| 6:00 AM - 2:00 PM | Kitchen \& Maint. staff in building |  | 7 |  |  |  |  |
| 6:30 AM - 4:00 PM | Teachers and Staff in Building | Daily M-F | 156 | 24 |  |  |  |
| 6:45 AM - 7:55 AM | Early Care Drop-off | Daily M-F |  | 10 |  |  | Indicates expected max cars at any one time. |
| 7:45 AM - 8:00 AM | Pre-K Parent Park \& Drop-Off Arrival | Daily M-F |  | 16 |  |  | Park \& Drop Lot assumes 16 live spaces with 2-3 minute use; additional vehicles can use signed north row of west parking lot |
| 8:00 AM - 8:15 AM | Parent Drop-Off \& Arrival | Daily M-F |  |  | 74 |  | Assume live spaces in a moving line; 74 vehicles at any one time |
| 8:00 AM - 2:30 PM | Parent Volunteers | Daily M-F | 4 |  |  |  |  |
| 8:00 AM - 4:00 PM | Itinerant Staff in Building | Daily M-F |  | 2 |  |  |  |
| 8:00 AM - 4:00 PM | Long Term Visitors | Daily M-F |  | 18 |  |  |  |
| 2:45 PM - $3: 15$ PM | Dismissal and Parent Pick-up | Daily M-F |  |  | 74 |  | Some parents may queue earlier than this; 74 vehicles at any one time, additional early cars may park in $\sim 89$ vacant site spaces. Dismissals will be staged to even out the peak flow of traffic. |
| AFTERNOON |  |  |  |  |  |  |  |
| 3:00 PM - 5:00 PM | Student Game - Soccer Fields | Spring/Fall M-F | 168 |  |  |  | (32 players [assume $50 \%$ car factor] +6 adults +6 additional spectators) X 6 soccer fields $=168$ cars |
| 3:00 PM - 5:00 PM | Student Game - Gymnasium | Winter M-F | 47 |  |  |  | Assumes basketball game: 20 players, 6 adults, 40 parents, 1 custd. |
| 3:00 PM - 5:00 PM | School Meetings - Faculty/Staff | Daily M-TH | 127 |  |  |  | Assume all-staff meeting (peak count), 1 custodian |
| 3:00 PM - 5:00 PM | School Club Meeting - Staff | 2 x per week | 5 |  |  |  | Assume 20 student members, 4 adults, 1 custodian |

## W.E. BALMER ELEMENTARY SCHOOL

## DORE \& WHITTIER ARCHITECTS

## DESIGN DEVELOPMENT ZONING SUBMISSION - PARKING ANALYSIS

February 28, 2019

${ }^{2}$ This number assumes a competition event with full bleachers. Most community sporting events in the gym will be much more sparsely attended.

# THE NEW W. EDV/ARD <br> NORTHBRIDGE, MASSACHUSETTS 



## PLANNING BOARD SUBMISSION

## APRIL 9, 2019

SMMA
Project Management


N ORTHBRIDGE
PUBLIC SCHOOLS

Massachusetts School Building Authority


E
FONTAINE BROS., INC.

## DD SITE \& LANDSCAPE DESIGN PLAN

- 246 parking spaces
- 74 parent drop-off queue spaces
- 20 car active drop off curb zone
- Bus queue: (10) 40 ' busses or (7) $40^{\prime}$ busses and (4) $30^{\prime}$ busses or vans











## W. EDWARD BALMER ELEMENTARY SCHOOL

DORE \& WHITTIER ARCHITECTS
SCALE: 1/2" = 1'-0"
LANDSCAPE SIGNAGE DETAIL - MAIN SITE ENTRANCE AT CRESCENT STREET

## OPTICAL HOUSING

Heavy cast low copper aluminum (A356 alloy; $<0.2 \%$ copper) assembly with integral cooling fins. The Optical Panel mounting surface is milled flat (surface variance <土 .002") to facilitate thermal transfer of heat to housing and cooling fins. Solid barrier wall separates optical and electrical compartments. The optical and electrical compartments are integrated to create one assembly. Minimum wall thickness is . $188^{\prime \prime}$.

## ELECTRICAL HOUSING w/ INTEGRATED ARM

Heavy cast low copper aluminum (A356 alloy; $<0.2 \%$ copper) assembly with integral cooling ribs surrounding the electrical compartment and a flat surface on the top of the arm to accommodate a photocell receptacle. Solid barrier wall separates optical and electrical compartments. The optical compartment and electrical compartment with the integrated support arm combine to create one assembly. Minimum wall thickness is .188". Cast and hinged driver assembly cover is integrated with wiring compartment cover.

## PLED"'OPTICS

Emitters (LED's) are arrayed on a metal core PCB panel with each emitter located on a copper thermal transfer pad and enclosed by an LED refractor. LED optics completely seal each individual emitter to meet an IP66 rating. In asymmetric distributions, a micro-reflector inside the refractor re-directs the house side emitter output towards the street side and functions as a house side shielding element. Refractors are injection molded H12 acrylic. Each LED refractor is sealed to the PCB over an emitter and all refractors are retained by an aluminum frame. Any one Panel, or group of Panels in a luminaire, have the same optical pattern. LED refractors produce standard site/area distributions. Panels are field replaceable and field rotatable in $90^{\circ}$ increments.

## LED DRIVER(S)

Constant current electronic with a power factor of $>.90$ and a minimum operating temperature of $-40^{\circ} \mathrm{F} /-40^{\circ} \mathrm{C}$. Driver(s) is/are UL and CUL recognized and mounted directly against the Electrical Housing to facilitate thermal transfer, held down by universal clamps to facilitate easy removal. In-line terminal blocks facilitate wiring between the driver and optical arrays. Drivers accept an input of $120-277 \mathrm{~V}, 50 / 60 \mathrm{~Hz}$ or $347 \mathrm{~V}-480 \mathrm{~V}, 50,60 \mathrm{~Hz}$. ( $0-10 \mathrm{~V}$ dimmable driver is standard. Driver has a minimum of 3 KV internal surge protection. Luminaire supplied with 20KV surge protector for field accessible installation.)

## LED EMITTERS

High output LED's are utilized with drive currents ranging from 350 mA to 1050 mA . 70 CRI Minimum. LED's are available in standard Neutral White (4000K), or optional Cool White (5000K) or Warm White (3000K). Consult Factory for other LED options.

AMBER LED's
PCA (Phosphor Converted Amber) LED's utilize phosphors to create color output similar to LPS Iamps and have a slight output in the blue spectral bandwidth. TRA (True Amber) LED's utilize material that emits light in the amber spectral bandwidth only without the use of phosphors.

## FINISH

Electrostatically applied TGIC Polyester Powder Coat on substrate prepared with 20 PSI power wash at $140^{\circ}$ F. Four step media blas $\dagger$ and iron phosphate pretreatment for protection and paint adhesion. $400^{\circ} \mathrm{F}$ bake for maximum hardness and durability.

MAST ARM FITTER/ELECTRICAL HOUSING
Replaces standard Electrical Housing. Fits standard 2 3/8" O.D. horizontal tenon. Two (2) straps with two (2) bolts each encircle the lower half of the tenon. Upper half of the tenon rests on self-centering steps that position the angle of the luminaire at $0^{\circ}$, $+1.5^{\circ},+1.5$ or $+3^{\circ}$ up from the horizontal. All hardware is stainless steel.

## PROJECT NAME:

## PROJECT TYPE:


*DLC PENDING AS OF 7/17


## RAZAR SERIES-LED



|  | Approximate Average Lumens - 4000K <br> (Lumens median of all distributions) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 350 mA |  |  | 525mA |  |  | 700 mA |  |  | 1050mA |  |  |
|  | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. |
| 24 | 28 | 3541 | 50 | 41 | 5058 | $\begin{aligned} & 70- \\ & 100 \end{aligned}$ | 53 | 6567 | 100 | 81 | 8773 | $150-$ |
| 40 | 45 | 5997 | $\begin{gathered} 70- \\ 100 \end{gathered}$ | 66 | 8653 | $\begin{aligned} & 100- \\ & 150 \end{aligned}$ | 87 | 10995 | 175 | 134 | 14647 | $\begin{aligned} & 200- \\ & 250 \end{aligned}$ |
| 48 | 55 | 7046 | 100 | 81 | 10018 | $\begin{aligned} & 150- \\ & 175 \end{aligned}$ | 105 | 12600 | 200 | 160 | 17566 | 250 |
| 80 | 87 | 11622 | $\begin{aligned} & 175- \\ & 200 \end{aligned}$ | 131 | 16736 | $200-$ | 174 | 21235 | 400 | 266 | 28190 | $\begin{aligned} & 450- \\ & 575 \end{aligned}$ |
| 120 | 127 | 17405 | 250 | 195 | 24860 | 450 | 260 | 31592 | $\begin{aligned} & 575- \\ & 750 \end{aligned}$ | 396 | 43323 | $\begin{array}{r} 750- \\ 1000 \end{array}$ |



Spec/Order Example: RZR/PLED-IV/80LED-700mA/CW/277/RAL-8019-S


| LED COUNT | $\begin{aligned} & \text { SOURCE } \\ & \text { TYPE } \end{aligned}$ | SOURCE | INITIAL LUMENS 4000 K CCT | INITIAL LUMENS 3000K CCT | INITIAL LUMENS 5000 K CCT | L70 GREATER THAN (HR) | STARTING TEMP. | SYSTEM <br> WATTS | VOLTS | MAX <br> INPUT AMPS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | LED | 24 PLED ${ }^{\circledR}$ Optical Module-350mA | $\begin{aligned} & 3,298- \\ & 3,784 \end{aligned}$ | $\begin{aligned} & 3,133- \\ & 3,595 \end{aligned}$ | $\begin{aligned} & 3,463 \\ & 3,973 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 29 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.24 \\ & 0.10 \end{aligned}$ |
| 24 | LED | 24 PLED ${ }^{\circ}$ Optical Module - 525mA | $\begin{aligned} & 4,711- \\ & 5,405 \end{aligned}$ | $\begin{aligned} & 4,475- \\ & 5,135 \end{aligned}$ | $\begin{aligned} & 4,947- \\ & 5,675 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 42 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.34 \\ & 0.15 \end{aligned}$ |
| 24 | LED | 24 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 6,023 \\ & 6,911 \end{aligned}$ | $\begin{aligned} & 5,722- \\ & 6,565 \end{aligned}$ | $\begin{aligned} & 6,324- \\ & 7,256 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 56 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.45 \\ & 0.20 \end{aligned}$ |
| 24 | LED | 24 PLED ${ }^{\circ}$ Optical Module - 1050mA | $\begin{aligned} & 8,171 \\ & 9,375 \end{aligned}$ | $\begin{aligned} & 7,762- \\ & 8,906 \end{aligned}$ | $\begin{aligned} & 8,580- \\ & 9,844 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 82 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.68 \\ & 0.30 \end{aligned}$ |
| 40 | LED | 40 PLED Optical Module - 350mA | $\begin{aligned} & 5,585- \\ & 6,408 \end{aligned}$ | $\begin{aligned} & 5,306- \\ & 6,088 \end{aligned}$ | $\begin{aligned} & 5,864- \\ & 6,729 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 43 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.38 \\ & 0.17 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circledR}$ Optical Module - 525mA | $\begin{aligned} & 8,059- \\ & 9,246 \end{aligned}$ | $\begin{aligned} & 7,656- \\ & 8,784 \end{aligned}$ | $\begin{aligned} & 8,462- \\ & 9,709 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 65 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.55 \\ & 0.24 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & \text { 10,240 - } \\ & 11,749 \end{aligned}$ | $\begin{aligned} & 9,728- \\ & 11,162 \end{aligned}$ | $\begin{aligned} & 10,752- \\ & 12,337 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 87 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.73 \\ & 0.32 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circledR}$ Optical Module - 1050mA | $\begin{aligned} & 13,642- \\ & 15,652 \end{aligned}$ | $\begin{aligned} & 12,960- \\ & 14,870 \end{aligned}$ | $\begin{aligned} & 14,324- \\ & 16,435 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 128 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.12 \\ & 0.49 \end{aligned}$ |
| 48 | LED | 48 PLED ${ }^{\circledR}$ Optical Module - 350mA | $\begin{aligned} & 6,562- \\ & 7,529 \end{aligned}$ | $\begin{aligned} & 6,234- \\ & 7,153 \end{aligned}$ | $\begin{aligned} & 6,890- \\ & 7,909 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 53 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.46 \\ & 0.20 \end{aligned}$ |
| 48 | LED | 48 PLED ${ }^{\circ}$ Optical Module-525mA | $\begin{aligned} & 9,330- \\ & 10,705 \end{aligned}$ | $\begin{aligned} & 8,864- \\ & 10,170 \end{aligned}$ | $\begin{aligned} & 9,797- \\ & 11,240 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 79 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.68 \\ & 0.29 \end{aligned}$ |
| 48 | LED | 48 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 11,735- \\ & 13,464 \end{aligned}$ | $\begin{aligned} & 11,148- \\ & 12,791 \end{aligned}$ | $\begin{aligned} & 12,322- \\ & 14,137 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 106 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.88 \\ & 0.38 \end{aligned}$ |
| 48 | LED | 48 PLED ${ }^{\circledR}$ Optical Module - 1050mA | $\begin{aligned} & 16,360- \\ & 18,771 \end{aligned}$ | $\begin{aligned} & 15,542- \\ & 17,832 \end{aligned}$ | $\begin{aligned} & 17,178-1 \\ & \text { 19,709 } \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 160 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.33 \\ & 0.58 \end{aligned}$ |
| RZR |  |  |  |  |  |  |  |  |  |  |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 350 mA | $\begin{aligned} & 10,824- \\ & 12,419 \end{aligned}$ | $\begin{aligned} & 10,283- \\ & 11,798 \end{aligned}$ | $\begin{aligned} & 11,365- \\ & 13,040 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 86 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.75 \\ & 0.33 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 525mA | $\begin{aligned} & 15,587- \\ & 17,884 \end{aligned}$ | $\begin{aligned} & 14,808- \\ & 16,990 \end{aligned}$ | $\begin{aligned} & 16,366- \\ & 18,778 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 130 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.10 \\ & 0.48 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 19,767- \\ & 22,680 \end{aligned}$ | $\begin{aligned} & 18,779- \\ & 21,546 \end{aligned}$ | $\begin{aligned} & 20,755- \\ & 23,814 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 174 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.45 \\ & 0.63 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 1050mA | $\begin{aligned} & 26,255- \\ & 30,124 \end{aligned}$ | $\begin{aligned} & 24,942- \\ & 28,618 \end{aligned}$ | $\begin{aligned} & 27,568- \\ & 31,630 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 257 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 2.22 \\ & 0.96 \end{aligned}$ |
| RZR-G |  |  |  |  |  |  |  |  |  |  |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 350 mA | $\begin{aligned} & 10,950- \\ & 12,564 \end{aligned}$ | $\begin{aligned} & 10,403- \\ & 11,936 \end{aligned}$ | $\begin{aligned} & 11,498- \\ & 13,192 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 87 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.75 \\ & 0.33 \end{aligned}$ |
| 80 | LED | 80 PLED Optical Module - 525mA | $\begin{aligned} & 15,735- \\ & 18,054 \end{aligned}$ | $\begin{aligned} & 14,948- \\ & 17,151 \end{aligned}$ | $\begin{aligned} & 16,522- \\ & 18,957 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 129 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.10 \\ & 0.48 \end{aligned}$ |
| 80 | LED | 80 PLED $^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 20,074- \\ & 23,032 \end{aligned}$ | $\begin{aligned} & \text { 19,071 - } \\ & 21,881 \end{aligned}$ | $\begin{aligned} & 21,078- \\ & 24,184 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 174 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.45 \\ & 0.63 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circledR}$ Optical Module - 1050mA | $\begin{aligned} & 27,651- \\ & 31,725 \end{aligned}$ | $\begin{aligned} & 26,268- \\ & 30,139 \end{aligned}$ | $\begin{aligned} & 29,033- \\ & 33,311 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 266 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 2.22 \\ & 0.96 \end{aligned}$ |
| 120 | LED | 120 PLED ${ }^{\circledR}$ Optical Module - 350 mA | $\begin{aligned} & 16,211- \\ & 18,599 \end{aligned}$ | $\begin{aligned} & 15,400- \\ & 17,669 \end{aligned}$ | $\begin{aligned} & \text { 17,021 - } \\ & \text { 19,529 } \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 130 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.06 \\ & 0.46 \end{aligned}$ |
| 120 | LED | 120 PLED ${ }^{\circ}$ Optical Module - 525 mA | $\begin{aligned} & 23,154- \\ & 26,566 \end{aligned}$ | $\begin{aligned} & 21996 \text { - } \\ & 25,238 \end{aligned}$ | $\begin{aligned} & 24,312- \\ & 27,894 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 192 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.63 \\ & 0.70 \end{aligned}$ |
| 120 | LED | 120 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 29,424- \\ & 33,760 \end{aligned}$ | $\begin{aligned} & 27,953- \\ & 32,072 \end{aligned}$ | $\begin{aligned} & 30,895- \\ & 35,448 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 260 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 2.17 \\ & 0.94 \end{aligned}$ |
| 120 | LED | 120 PLED ${ }^{\circ}$ Optical Module - 1050mA | $\begin{aligned} & 40,350- \\ & 46,296 \end{aligned}$ | $\begin{aligned} & 38,333- \\ & 43,981 \end{aligned}$ | $42,368-$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 398 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 3.33 \\ & 1.43 \end{aligned}$ |

NOTES: 1. Max Input Amps is the highest of starting, operating, or open circuit currents.
2. Lumen values for LED Modules vary according to the distribution type. 80LED array appears in both the RZR and RZR-G models.
3. System Watts includes the source watts and all driver components.
4. Fuse value should be sufficient to protect all wiring components. For electronic driver and LED component protection, use surge suppressor supplied with luminaire Note: Surge suppressors are considered a perishable device.
5. L7O(10K) - TM-21 $6 x$ rule applied.

WARNING: All fixtures must be installed in accordance with local codes or the National Electrical Code. Failure to do so may result in serious personal injury.

## sOLID STATE AREA LIGHTING

## RAZAR-PT2 SEZTIES-PLED

S P E C I F I C A T I O N S

## OPTICAL HOUSING

Heavy cast low copper aluminum (A356 alloy; <0.2\% copper) assembly with integral cooling fins. The Optical Panel mounting surface is milled flat (surface variance $< \pm .003$ ") to facilitate thermal transfer of heat to housing and cooling fins. Minimum wall thickness is $.188^{\prime \prime}$. All hardware is stainless steel

TWIN ARM POST TOP MOUNTING/ELECTRICAL COMPARTMENT Two (2) 1/2" Sch. 40 round aluminum arms are welded to a cast low copper aluminum (A356 alloy; $<0.2 \% \mathrm{Cu}$ ) pole top tenon fitter which also serves as the LED Driver and wiring compartment. Tenon maximum $27 / 8^{\prime \prime}$ diameter x $31 / 2^{\prime \prime}$ height. All exposed hardware is stainless steel.

## PLED" ${ }^{\text {m" }}$ OPTICS

Emitters (LED's) are arrayed on a metal core PCB panel with each emitter located on a copper thermal transfer pad and enclosed by an LED refractor. LED optics completely seal each individual emitter to meet an IP66 rating. In asymmetric distributions, a micro-reflector inside the refractor re-directs the house side emitter output towards the street side and functions as a house side shielding element. Refractors are injection molded H 12 acrylic. Each LED refractor is sealed to the PCB over an emitter and all refractors are retained by an aluminum frame. Any one Panel, or group of Panels in a luminaire, have the same optical pattern. LED refractors produce standard site/area distributions. Panels are field replaceable and field rotatable in $90^{\circ}$ increments.

## LED DRIVERS

Constant current electronic with a power factor of $>.90$ and a minimum operating temperature of $-40^{\circ} \mathrm{F} /-40^{\circ} \mathrm{C}$. Driver(s) is/are UL and cUL recognized and mounted directly against the Electrical Housing to facilitate thermal transfer, held down by universal clamps to facilitate easy removal. In-line terminal blocks facilitate wiring between the driver and optical arrays. Drivers accept an input of $120-277 \mathrm{~V}, 50 / 60 \mathrm{~Hz}$ or $347 \mathrm{~V}-480 \mathrm{~V}$, $50,60 \mathrm{~Hz}$. ( $0-10 \mathrm{~V}$ dimmable driver is standard. Driver has a minimum of 3 KV internal surge protection. Luminaire supplied with 20 KV surge protector for field accessible installation.)

## AMBER LED's

PCA (Phosphor Converted Amber) LED's utilize phosphors to create color output similar to LPS lamps and have a slight output in the blue spectral bandwidth. TRA (True Amber) LED's utilize material that emits light in the amber spectral bandwidth only without the use of phosphors.

## FINISH

Electrostatically applied TGIC Polyester Powder Coat on substrate prepared with 20 PSI power wash at $140^{\circ} \mathrm{F}$. Four step sand blast and iron phosphate pretreatment for protection and paint adhesion. $400^{\circ} \mathrm{F}$ bake for maximum hardness and durability. Texture finish is standard.


PATENT PENDING


## RZZZR-PT2 SERIEES-PLED

## S P E C I F I C A T I O N S

## PLED ${ }^{\text {T" }}$ MODULES



Approximate Average Lumens - 4000K
(Lumens median of all distributions)

|  | 350 mA |  |  | 525 mA |  |  |  | 700 mA |  |  | 1050mA |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. | Watts | Lumens | HID Eq. |  |
| $\mathbf{4 0}$ | 45 | 5997 | $70-$ <br> 100 | 66 | 8653 | $100-$ | 87 | 10995 | 175 | 134 | 14647 | $200-$ |  |
| $\mathbf{1 5 0}$ | 87 | 11622 | $175-$ | 131 | 16736 | $200-$ | 174 | 21235 | 400 | N/A | N/A | N/A |  |

Spec/Order Example: RZR-PT2-LED/PLED-V-SQ/80LED-700mA/NW/277/RAL9005

| S | E C / O | R D | E P | N G | N F | R M | T \\| ○ N |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MODEL | OPTICS | LED MODE |  |  | VOLTAGE | FINISH | OPTIONS |
| MODEL | OPTICS | LED MODE |  |  | VOLTAGE | FINISH | OPTIONS |
|  |  | NO. LEDs |  | COLOR TEMP-CCT | $\begin{aligned} & \square 120 \\ & \square 208 \\ & \square 240 \\ & \square 277 \\ & \square 347 \\ & \square 480 \end{aligned}$ | STANDARD TEXTURED FINISH |  |
| $\square$ RZR-PT2 |  |  |  |  |  | BLACK RAL-9005-T WHITE <br> RAL-9003-T GREY <br> RAL-7004-T DARK BRONZE RAL-8019-T GREEN RAL-6005-T <br> FOR SMOOTH FINSHREPLACE SUFFX "T" <br> WTH SUFFIX's" <br> " (EXAMPLE: RAL-9005-S) CONSULT FACTORY FOR CUSTOM COLORS |  |
| U.S. Architectural Lighting |  | 660 West Avenue O, Palmadele, CA 93551 Phone (661) 233-2000 Fax (661) 233-200 www.usaltg.com |  |  |  |  |  |


| $\begin{aligned} & \text { LED } \\ & \text { COUNT } \end{aligned}$ | SOURCE TYPE | SOURCE | INITIAL LUMENS 4000 K CCT | INITIAL LUMENS 3000K CCT | INITIAL LUMENS 5000K ССT | L70 GREATER THAN (HR) | STARTING TEMP. | SYSTEM WATTS | VOLTS | MAX INPUT AMPS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 40 | LED | 40 PLED Optical Module - 350 mA | $\begin{aligned} & 5,585- \\ & 6,408 \end{aligned}$ | $\begin{aligned} & 5,306- \\ & 6,088 \end{aligned}$ | $\begin{aligned} & 5,864- \\ & 6,729 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 45 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.38 \\ & 0.17 \end{aligned}$ |
| 40 | LED | 40 PLED Optical Module-525mA | $\begin{aligned} & 8,059- \\ & 9,246 \end{aligned}$ | $\begin{aligned} & 7,656- \\ & 8,784 \end{aligned}$ | $\begin{aligned} & 8,462- \\ & 9,709 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 66 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.55 \\ & 0.24 \end{aligned}$ |
| 40 | LED | 40 PLED Optical Module - 700mA | $\begin{aligned} & \text { 10,240- } \\ & 11,749 \end{aligned}$ | $\begin{aligned} & 9,728- \\ & 11,162 \end{aligned}$ | $\begin{aligned} & \text { 10,752 - } \\ & 12,337 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 87 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.73 \\ & 0.32 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circ}$ Optical Module - 1050mA | $\begin{aligned} & 13,642- \\ & 15,652 \end{aligned}$ | $\begin{aligned} & 12,960- \\ & 14,870 \end{aligned}$ | $\begin{aligned} & 14,324- \\ & 16,435 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 134 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.12 \\ & 0.49 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circ}$ Optical Module - 350 mA | $\begin{aligned} & 10,824- \\ & 12,419 \end{aligned}$ | $\begin{aligned} & \text { 10,283-} \\ & 11,798 \end{aligned}$ | $\begin{aligned} & 11,365- \\ & 13,040 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 87 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 0.75 \\ & 0.33 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circ}$ Optical Module - 525mA | $\begin{aligned} & 15,587- \\ & 17,884 \end{aligned}$ | $\begin{aligned} & 14,808- \\ & 16,990 \end{aligned}$ | $\begin{aligned} & 16,366- \\ & 18,778 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 131 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.10 \\ & 0.48 \end{aligned}$ |
| 80 | LED | 80 PLED ${ }^{\circ}$ Optical <br> Module - 700mA | $\begin{aligned} & 19,767- \\ & 22,680 \end{aligned}$ | $\begin{aligned} & 18,779- \\ & 21,546 \end{aligned}$ | $\begin{aligned} & 20,755- \\ & 23,814 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 174 | $\begin{aligned} & 120 \\ & 277 \end{aligned}$ | $\begin{aligned} & 1.45 \\ & 0.63 \end{aligned}$ |

NOTES:

1. Max Input Amps is the highest of starting, operating, or open circuit currents
2. Lumen values for LED Modules vary according to the distribution type
3. System Watts includes the source watts and all driver components.
4. Fuse value should be sufficient to protect all wiring components. For electronic driver and LED component protection, use surge suppressor supplied with luminaire. Note: Surge suppressors are considered a perishable device.
5. L70(10K) - TM-21 $6 x$ rule applied

WARNING: All fixtures must be installed in accordance with local codes or the National Electrical Code. Failure to do so may result in serious personal injury.


SOLID STATE AREA LIGHTING

## 

OPTICAL HOUSING
Heavy cast low copper aluminum (A356 alloy: <0.2\% copper) assembly with integral cooling fins. The Optical Panel mounting surface is milled flat (surface variance < $\pm .003^{\prime \prime}$ ) to facilitate thermal transfer of heat to housing and cooling fins. The Optical Housing bolts to the Electrical Housing forming a unified assembly. The minimum wall thickness is .188".

## ELECTRICAL HOUSING

Heavy cast low copper aluminum (A356 alloy; $<0.2 \%$ copper) assembly. Minimum wall thickness is .188". Fixture Mounting Plate affixes to mounting surface over a recessed j-box. Electrical Housing anchors on the top edge of the Mounting Plate and stainless steel recessed socket head screws tighten the Electrical Housing to the Mounting Plate from the bottom.

## PLED"' OPTICAL MODULES

Emitters (LED's) are arrayed on a metal core PCB panel with each emitter located on a copper thermal transfer pad and enclosed by an LED refractor. LED optics completely seal each individual emitter to meet an IP66 rating. The asymmetric distributions, have a micro-reflector inside the refractor which re-directs the house side emitter output towards the street side and functions as a house side shielding element. Refractors are injection molded H12 acrylic. Each LED refractor is sealed to the PCB over an emitter and all refractors are retained by an aluminum frame. Any one Panel, or group of Panels in a luminaire, have the same optical pattern. LED refractors produce Type II, III, and Type IV site/area distributions as well as other specialty asymmetric distributions. Panels are field replaceable and field rotatable in $90^{\circ}$ increments.

## LED DRIVER(S)

Constant current electronic with a power factor of $>.90$ and a minimum operating temperature of $-40^{\circ} \mathrm{F} /-40^{\circ} \mathrm{C}$. Driver (s) is/are UL and cUL recognized and mounted directly agains $\dagger$ the Electrical Housing to facilitate thermal transfer, held down by universal clamps to facilitate easy removal. In-line terminal blocks facilitate wiring between the driver and optical arrays. Drivers accept an input of $120-277 \mathrm{~V}, 50 / 60 \mathrm{~Hz}$ or $347 \mathrm{~V}-480 \mathrm{~V}$, $50,60 \mathrm{~Hz}$. ( $0-10 \mathrm{~V}$ dimmable driver is standard. Driver has a minimum of 3 KV internal surge protection. Luminaire supplied with 20 KV surge protector for field accessible installation.)

## LED EMITTERS

High output LED's are utilized with drive currents ranging from 350 mA to 1050 mA . 70CRI Minimum. LED's are available in standard Neutral White (4000K), or optional Cool White (5000K) or Warm White (3000K). Consult Factory for other LED options.

AMBER LED's
PCA (Phosphor Converted Amber) LED's utilize phosphors to create color output similar to LPS lamps and have a slight output in the blue spectral bandwidth. TRA (True Amber) LED's utilize material that emits light in the amber spectral bandwidth only without the use of phosphors.

## FINISH

Electrostatically applied TGIC Polyester Powder Coat on substrate prepared with 20 PSI power wash at $140^{\circ} \mathrm{F}$. Four step media blast and iron phosphate pretreatment for protection and paint adhesion. $400^{\circ} \mathrm{F}$ bake for maximum hardness and durability.


| FIXTURE | A | B | C |
| :---: | :---: | :---: | :---: |
| RZRW1 | ${ }_{\substack{\text { chenm" } \\ \text { (22mm) }}}$ | ${ }_{\text {(205mm) }}^{12}$ | ${ }_{(152 m m)}^{60}$ |
| RZRW1-EM |  | ${ }_{(356 \mathrm{~mm})}^{14}$ | ${ }_{(6.55 \mathrm{~mm}}{ }^{6.5}$ |

## RZR-WM1

PATENT PENDING


| FIXTURE | A | B | C |
| :---: | :---: | :---: | :---: |
| RZRW2 | $16^{\prime \prime}$ <br> $(406 \mathrm{~mm})$ | $122^{\prime \prime}$ <br> $(305 \mathrm{~mm})$ | $6^{\prime \prime}$ <br> $(152 \mathrm{~mm})$ |
| RZRW2-EM | $16^{\prime \prime}$ <br> $(406 \mathrm{~mm})$ | $14^{\prime \prime}$ <br> $(356 \mathrm{~mm})$ | 6.5 E <br> $(165 \mathrm{~mm})$ |

## RZR-WM2

PATENT PENDING


RZR-WM3
PATENT PENDING
-
$\square \rightarrow$

'---------- LIGHTING'

## RAZAR WALLMOUNT SERIES-LED



Spec/Order Example: RZR-WM2/PLED-IV/40LED-700mA/CW/277/RAL-8019-S/SF


| LED COUNT | $\begin{aligned} & \text { SOURCE } \\ & \text { TYPE } \end{aligned}$ | SOURCE | INITIAL LUMENS 4000K | INITIAL LUMENS 3000K | INITIAL LUMENS 5000K | L70 GREATER <br> THAN (HR)-TM21 | STARTING TEMP. | SYSTEM WATTS | VOLTS | MAX INPUT AMPS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 20 | LED | 20 PLED ${ }^{\circledR}$ Optical Module - 350mA | $\begin{aligned} & 2,706- \\ & 2,993 \end{aligned}$ | $\begin{aligned} & 2,571- \\ & 2,843 \end{aligned}$ | $\begin{aligned} & 2,841- \\ & 3,143 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 22 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.19 \\ & 0.08 \\ & 0.07 \end{aligned}$ |
| 20 | LED | 20 PLED Optical Module - 525mA | $\begin{aligned} & 3,897- \\ & 4,310 \end{aligned}$ | $\begin{aligned} & 3,702- \\ & 4,095 \end{aligned}$ | $\begin{aligned} & 4,092- \\ & 4,526 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 33 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.28 \\ & 0.12 \\ & 0.10 \end{aligned}$ |
| 20 | LED | 20 PLED ${ }^{\circledR}$ Optical Module - 700mA | $\begin{aligned} & 4,942- \\ & 5,466 \end{aligned}$ | $\begin{aligned} & 4,695- \\ & 5,193 \end{aligned}$ | $\begin{aligned} & 5,189- \\ & 5,739 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 44 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.37 \\ & 0.16 \\ & 0.13 \end{aligned}$ |
| 20 | LED | 20 PLED ${ }^{\circ}$ Optical Module - 1050mA | $\begin{aligned} & 6,564 \text { - } \\ & 7,260 \end{aligned}$ | $\begin{aligned} & 6,236- \\ & 6,897 \end{aligned}$ | $\begin{aligned} & \text { 6,892 - } \\ & 7,623 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 65 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.55 \\ & 0.24 \\ & 0.19 \end{aligned}$ |
| 40 | LED | 40 PLED Optical Module - 350mA | $\begin{aligned} & 5,585- \\ & 6,178 \end{aligned}$ | $\begin{aligned} & 5,206- \\ & 5,869 \end{aligned}$ | $\begin{aligned} & 5,864- \\ & 6,487 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 43 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.36 \\ & 0.16 \\ & 0.13 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circ}$ Optical Module - 525mA | $\begin{aligned} & 8,059- \\ & 8,914 \end{aligned}$ | $\begin{aligned} & 7,656- \\ & 8,468 \end{aligned}$ | $\begin{aligned} & 8,462- \\ & 9,360 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 65 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.55 \\ & 0.24 \\ & 0.19 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\text {® }}$ Optical Module - 700mA | $\begin{aligned} & 10,240- \\ & 11,327 \end{aligned}$ | $\begin{aligned} & 9,728- \\ & 10,761 \end{aligned}$ | $\begin{aligned} & 10,752- \\ & 11,893 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 87 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.73 \\ & 0.32 \\ & 0.26 \end{aligned}$ |
| 40 | LED | 40 PLED ${ }^{\circ}$ Optical Module - 1050mA | $\begin{aligned} & 13,642- \\ & 15,089 \end{aligned}$ | $\begin{aligned} & 12,690- \\ & 14,335 \end{aligned}$ | $\begin{aligned} & 14,324- \\ & 15,843 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 129 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 1.08 \\ & 0.47 \\ & 0.38 \end{aligned}$ |
| 60 | LED | 60 PLED ${ }^{\circ}$ Optical Module - 350mA | $\begin{aligned} & 8,118- \\ & 8,979 \end{aligned}$ | $\begin{aligned} & 7,712- \\ & 8,530 \end{aligned}$ | $\begin{aligned} & 8,524 \text { - } \\ & 9,428 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 65 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.55 \\ & 0.24 \\ & 0.19 \end{aligned}$ |
| 60 | LED | 60 PLED ${ }^{\circ}$ Optical Module - 525mA | $\begin{aligned} & 11,690- \\ & 12,930 \end{aligned}$ | $\begin{aligned} & 11,106- \\ & 12,284 \end{aligned}$ | $\begin{aligned} & 12,275- \\ & 13,577 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 98 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 0.82 \\ & 0.36 \\ & 0.29 \end{aligned}$ |
| 60 | LED | 60 PLED Optical Module - 700mA | $\begin{aligned} & 14,825- \\ & 16,398 \end{aligned}$ | $\begin{aligned} & 14,084- \\ & 15,578 \end{aligned}$ | $\begin{aligned} & 15,566- \\ & 17,218 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 131 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 1.09 \\ & 0.47 \\ & 0.38 \end{aligned}$ |
| 60 | LED | 60 PLED Optical Module - 1050mA | $\begin{aligned} & 19,691- \\ & 21,780 \end{aligned}$ | $\begin{aligned} & 18,706- \\ & 20,691 \end{aligned}$ | $\begin{aligned} & 20,676- \\ & 22,869 \end{aligned}$ | 60,000+ | $-20^{\circ} \mathrm{F}$ | 193 | $\begin{aligned} & 120 \\ & 277 \\ & 347 \end{aligned}$ | $\begin{aligned} & 1.61 \\ & 0.70 \\ & 0.56 \end{aligned}$ |

## NOTES:

1. Max Input Amps is the highest of starting, operating, or open circuit currents
2. Lumen values for LED Modules vary according to the distribution type
3. System Watts includes the source watts and all driver components.
4. Fuse value should be sufficient to protect all wiring components.
5. L7O(10K) - TM-21 $6 x$ rule applied

L70(10K) - Calculated $=244,000 @ 700 \mathrm{~mA}$
$=102,000 @ 1050 \mathrm{~mA}$
WARNING: All fixtures must be installed in accordance with local codes or the National Electrical Code. Failure to do so may result in serious personal injury.

## SLV4

# BIRCHWOOD a LEVITON. VANESSA LED <br> lighting company <br> Wet Location Luminaire <br> Distributed Array LED 

Ceiling Mount | Wall Mount | CSS


VANESSA combines high-end architectural styling with precision engineering to create a strong, elegant wet-location luminaire designed to complement wet exterior or interior installations.

Weather-sealing prevents water and moisture from entering the lens, power entry points and end-caps. Constructed of heavy gauge extruded aluminum, precision machined smooth end-caps and extruded acrylic lenses, VANESSA is built to last while withstanding elements associated with wet-location applications.

VANESSA is available as LED and single or double T5 or T 8 , or single T 5 HO linear fluorescent lamps. $2^{\prime}, 3^{\prime}, 4^{\prime}$, $6^{\prime}$ and $8^{\prime}$ nominal lengths are standard, continuous runs are available.

UL and c-UL listed for wet locations.

Made in the USA

## LED Light Engine System

LED Light Engines are available as HLO (High Lumen Output) and SLO (Standard Lumen Output) providing efficient illumination. CLO (Custom Lumen Output) allows for end user specified lumen output or tailored wattage consumption for certain models. Consult factory for details.

## Dimming

Dimming is available with a variety of control protocols and options. Consult factory for availability and specifications.

## Acrylic Lens Options

(FW) Frosted White impact resistant extruded lens.

## Fixture Length

Fixtures are available in $2^{\prime}, 3^{\prime}, 4^{\prime}, 6^{\prime}$ and $8^{\prime}$ nominal lengths. Continuous run mounting available featuring water-sealed gaskets within knock-outs for maintaining WL rating. See installation section for more details.

## Custom and Mods

We proudly specialize in manufacturing custom and modified luminaires and have the ability to modify most of our standard fixtures. Please contact factory with any inquiries.

## Mounting Options

(CSS) Cable Suspension System field adjustable $1 / 16^{\prime \prime}$ aircraft cable, (WM) Wall Mount, (REC) Recessed or (CM) Ceiling Mount.

## Finishes

(SL) Silver Matte Texture, (MW) Matte White, or (FB) Flat Black. Other powder coat finishes available. Consult factory for details.

## VANESSA LED <br> Wet Location Luminaire Distributed Array LED



## CSS Options

| Cable Length | Power Cord Color |
| :--- | :--- |
| $36-36^{\prime \prime}$ (standard) | W - White |
| $72-72^{\prime \prime}$ | B - Black |
| $120-120^{\prime \prime}$ | G - Gray (standard) |



## NOTES

1 specify length in nominal feet
2 contact factory for custom finish
3 see options for non-standard selections
4 available for EB, D1, D10 drivers only
5 EM's are remote mounted along with the test switch in a dry location, consult factory for more info
6 6' \& 8' lengths are made up with (2) 3' fixtures or (2) 4' fixtures respectively
7 direct (DR) distribution only

VANESSA is rated for operation with ambient temperatures not to exceed $40^{\circ} \mathrm{C}$. Use specification code "HAT" for applications where ambient will be between $40^{\circ}$ and $45^{\circ} \mathrm{C}$. The "HAT" option is a thermistor which will control internal temperatures so as not to exceed internal device maximum temperature. At certain temperature thresholds, fixture will dim light output to keep internal temperatures within the acceptable range. Available for EB, D1 and D10 drivers only, consult factory for more details.
*see option sheet for details

LED

VAN-LED-400 (4000K FW lens)
SLO - $89.4 \mathrm{~lm} /$ watt delivered @ $4.4 \mathrm{w} / \mathrm{ft}$ consumed watts, $393 \mathrm{~lm} / \mathrm{ft}$.
HLO - $84 \mathrm{Im} /$ watt delivered @ $9 \mathrm{w} / \mathrm{ft}$ consumed watts, $756 \mathrm{Im} / \mathrm{ft}$.
CLO - refer to CLO Calculator
LED supplement info

## Transportation Impact Report

# W. Edward Balmer Elementary School Northbridge, MA 

January 26, 2018
Prepared for:
Dore \& Whittier Architects, Inc. 260 Merrimac Street, Bldg. 7

Newburyport, MA 01950

Submitted by:
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Nitsch Project \#12260.

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## INTRODUCTION

Nitsch Engineering has been retained by Dore \& Whittier Architects to prepare a qualitative assessment of safety, traffic circulation, and traffic access/egress, associated with the feasibility study and schematic design for the proposed W. Edward Balmer (Balmer) Elementary School reconstruction project located in Northbridge, Massachusetts.

The Project includes construction of a new Balmer Elementary School building and grounds on the site of the existing school, located at 21 Crescent St in Northbridge. The existing Balmer School is an elementary educational institution with an enrollment of 569 students in second through fourth grades, and approximately 49 staff.

The following four (4) potential options are being considered:

1. Option B2, Grades $2-4$, New Construction at the Rear of the Site ( 510 enrollment and approximately 49 staff)
2. Option C2, Addition/Renovation of the existing Balmer School, keeping the Academic Wing ( 1,030 enrollment K-5, plus $80 \mathrm{PK}, 1,110$ total and approximately 80staff)
3. Option C3, Grades PK-5, New Construction at the Rear of the Site ( 1,030 enrollment K-5, plus 80 PK, 1,110 total and approximately 80 staff)
4. Option C5, Grades PK-5, New Construction, Front of the Site ( 1,030 enrollment K-5, plus 80 PK, 1,110 total and approximately 80 staff)

The evaluation will be based on the two (2) enrollment options of A with 510 and $B$ with 1,110 students, because all four (4) options have identical access and egress points. In Option A, the existing school will be replaced in kind with a new school. In Option B., the new larger school will combine Balmer and Northbridge Elementary Schools.

The report describes the project area, presents traffic counts (taken in 2017), and evaluates the existing facilities and the site improvements to support the development alternatives by analyzing existing and future traffic operating efficiency. The data is used to determine the traffic circulations, overall operations, and to evaluate the traffic impacts of the proposed school.

The standards used for analysis conform to the 2009 edition of the Manual on Uniform Traffic Control Devices ${ }^{1}$ (MUTCD), 2009 edition and the 2010 edition of the Highway Capacity Manual.

The following conditions are analyzed in this report:

- Existing Conditions 2017;
- Future 2024 No-Build;
- Future 2024 Build based on enrollment option of 510 students; and
- Future 2024 Build based on enrollment option of 1,110 students.

Figure 1 is the Locus Map showing the proximity of the new school and the surrounding roadway network. Figure 2 shows the existing conditions of the school site.

[^1]

Figure 1: Locus Map

## W. Edward Balmer and Northbridge Elementary School Northbridge, Massachusetts



Figure 2: Existing Conditions

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts
### 2.1 Study Area Roadways

To examine the existing conditions, we studied and collected data at the following roadways:

1. Main Street,
2. North Main Street,
3. Crescent Street, and
4. Lake Street,

## Main Street

Main Street is classified by the Massachusetts Department of Transportation (MassDOT) as a Rural Major Collector and runs in the east-west directions between the Worcester-Providence Turnpike (Route 146) in Northbridge and Hill St in Northbridge. The posted speed limits along the roadway in the study area are 30 miles per hour. The land use is primarily commercial within the study area. The roadway is within the jurisdiction of the Town of Northbridge.

## North Main Street

North Main Street is classified by MassDOT as a Rural Major Collector and runs in the southeast-northwest directions between Goldthwaite Road and Main Street in Northbridge. The posted speed limits along the roadway in the study area are 25 miles per hour. The land use is primarily residential. The roadway is within the jurisdiction of the Town of Northbridge.

## Crescent Street

Crescent Street is classified by MassDOT as a local roadway and runs in the northeast-southwest directions between Mason Road and North Main Street in Northbridge. The roadway within the study area is designated as School Zone, with 20 miles per hour posted speed limits. The land use is primarily residential. The roadway is within the jurisdiction of the Town of Northbridge.

Lake Street
Lake Street is classified by MassDOT as a local roadway and runs in the north-south directions between Crescent Street and Main Street in Northbridge. The roadway within the study area does not have a posted speed limit. The roadway is within the jurisdiction of the Town of Northbridge.

### 2.2 Study Area Intersections

To examine the existing conditions, we included the following intersections in the study area. The intersection locations are shown in Figure 3.

1. Main Street at Lake Street
2. Main Street at North Main Street
3. North Main Street at Crescent Street,
4. Crescent Street at Arcade Street,
5. Crescent Street at Balmer Elementary School Driveway, and
6. Crescent Street at Lake Street.

Main Street at Lake Street
Main Street intersects Lake Street at a three-way unsignalized intersection with Main Street approaching from the east and west and Lake Street approaching from the north. Main Street operates freely with no control. Lake Street operates with stop control.

From both approaches Main Street is a two-way roadway with one travel lane in each direction separated with double yellow centerlines, and is approximately 30 feet wide. Approaching from the north Lake Street is approximately 26 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Continuous concrete sidewalks are present on both sides of each approach. A crosswalk is present across Main Street.

## Main Street at North Main Street

Main Street and North Main Street intersect as a three-way unsignalized intersection, with Main Street approaching from the west and east and North Main Street approaching from northwest. Main Street operates freely with no control. North Main Street operates with stop control.

From both approaches, Main Street is a two-way roadway with one lane in each direction, separated by double yellow centerline and is approximately 40 feet wide. Approaching from northwest, North Main Street is a twolane roadway separated with a double yellow center line. At the intersection, North Main Street is separated with a raised concrete median, and is approximately 72 feet wide. Cement concrete sidewalks are present along both sides of Main Street and North Main Street on the approach to the intersection. Crosswalks are present across North Main Street.

## North Main Street at Crescent Street

North Main Street, Crescent Street and C Street intersect at a four-way unsignalized intersection, with North Main Street approaching from southeast and northwest, Crescent Street approaching from the northeast, and C Street approaching from the southwest. Crescent Street and C Street operate with stop control. North Main Street operates freely with no control.

From both approaches North Main Street is a two-way roadway with one lane in each direction, separated by double yellow centerline. Approaching the intersection, the North Main Street is approximately 27 feet wide. Approaching the intersection Crescent Street and C Street are two-way roadways with one lane in each direction without separation and approximately 25 feet wide at the intersection. Cement concrete sidewalk and crosswalks are present on all sides of the intersection. Crosswalks are present across all approaches.

Crescent Street intersects Arcade Street at a three-way unsignalized intersection with Crescent Street approaching from the northeast and southwest and Arcade Street approaching from the southeast. Crescent Street operates freely with no control. Arcade Street operates with stop control.

From both approaches, Crescent Street is approximately 26 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Approaching from southeast, Arcade Street is approximately 25 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Continuous cement concrete sidewalks are present on both sides of Crescent Street. Continuous bituminous concrete sidewalks are present on both sides of Arcade Street. A Crosswalk is present across Arcade Street approach.

## Crescent Street at Balmer Elementary School Driveway

Crescent Street intersects the Balmer Elementary School Driveway at a three-way unsignalized intersection with Crescent Street approaching from the northeast and southwest and the driveway approaching from the northwest. Crescent Street operates freely with no control. The Balmer Elementary School Driveway operates with stop control.

From both approaches, Crescent Street is approximately 26 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Approaching from northwest, The Balmer Elementary School Driveway is approximately 30 feet wide. and contains one travel lane in each direction. There are no pavement markings separating the lanes. Continuous cement concrete sidewalks are present on both sides of Crescent Street. A continuous bituminous concrete sidewalk is present on easterly side of the driveway. Crosswalks are present across the Balmer Elementary School Driveway and the southwest Crescent Street approach leg.

## Crescent Street at Lake Street

Crescent Street intersects Lake Street at a three-way unsignalized intersection with Crescent Street approaching from the northeast and southwest and Lake Street approaching from the southeast. Crescent Street operates freely with no control. Lake Street operates with stop control.

From both approaches Crescent Street is approximately 30 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Lake Street is approximately 26 feet wide and contains one travel lane in each direction. There are no pavement markings separating the lanes. Cement concrete sidewalk is present on both sides of Crescent Street and the east side of Lake Street. Crosswalks are present across the Lake Street.


Figure 3: Intersection Locations

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts
### 2.3 Balmer Elementary School Site Visit

Nitsch Engineering conducted two site visits (Tuesday September 12, and Wednesday September 13, 2017) to observe the site circulation associated with the weekday morning drop-off, weekday afternoon pick-up and general queue lengths around both Balmer Elementary and Northbridge Elementary School sites. The weekday morning drop-off observation occurred during clear conditions with a temperature of approximately 72 degrees. The weekday afternoon pick-up activity occurred during clear conditions with a temperature of approximately 82 degrees.

### 2.4 Balmer Elementary School Site Access and Egress

Balmer Elementary School is located at 21 Crescent St, North of Main Street, in Northbridge. The School is accessed from Crescent Street. The access and egress to the school (parental drop-off and pick-up, as well as the teachers and staff) occurs from Crescent Street through the school driveway to the parking lot. The school driveway is approximately 590 feet long and 40 feet wide. An 8 -foot wide sidewalk is present at the easterly side of the driveway, which connects the sidewalk along Crescent Street to Balmer Elementary School.

### 2.5 Balmer Elementary School Traffic Circulation and Pick-up/Drop-off

Figures 4 and 5 graphically depicts the queuing activity during the weekday morning drop-off and afternoon pick-up periods at the existing Balmer Elementary School.

## Existing Morning Drop-off Circulation:

Parents arrive at the school through Crescent Street from 7:45 AM through 8:30 AM, and enter the two drop off lines at the fenced in play lot. The children are greeted by a couple of the teachers who assist them when exiting the cars. Some parents also park at the school lot and walk their children to the school entrance. A total of 112 vehicles entered the school parking lot, of which 74 parental drop-offs were observed during morning. A total of 17 buses and one mini-bus/Special Ed bus drop off students at the school. At the time of observation, we did not notice any bus and vehicular traffic conflict occurring. 71 vehicles entering the site were traveling southwest-bound on Crescent Street while 41 vehicles were traveling eastbound. 41 vehicles exiting the site were observed traveling eastbound on Crescent Street while 33 vehicles were observed traveling westbound.

## Existing Afternoon Pick-up Circulation:

The afternoon pick-up period occurs approximately from 1:45 PM to 2:45 PM. Parents start arriving from Crescent Street around 1:45 PM, and park at the fenced in play lot, southeast of the school, and wait for their children. We observed 63 vehicles parked at this lot. At the time of observation, we did not notice any parental vehicle parking extend out of the fenced area. All the parents walk to the school to collect their children at the Parent Pickup door. Once they have collected their children they leave via Crescent Street, and normal traffic returns around 2:45 PM. 45 vehicles entering the site were traveling westbound on Crescent Street while 18 vehicles were traveling eastbound. 42 vehicles exiting the site were observed traveling eastbound on Crescent Street while 44 vehicles were observed traveling westbound.

Table 1 quantifies the parent and bus drop-off/pick-up totals for the school during the site visit.

Table 1 - Balmer School Drop-Off/Pick-Up Quantity

| Type | Parent |  | Bus |  | Mini-Bus/SP. ED |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Drop-Off | Pick-Up | Drop-Off | Pick-Up | Drop-Off | Pick-Up |
| 7:45-8:00 | 2 |  | 3 |  |  |  |
| $8: 00-8: 15$ | 40 |  | 11 |  |  |  |
| 8:15-8:30 | 32 |  | 3 |  | 1 |  |
| 1:45-2:00 |  | 4 |  | 1 |  |  |
| $2: 00-2: 15$ |  | 20 |  | 2 |  |  |
| $2: 15-2: 30$ |  | 27 |  | 4 |  |  |
| $2: 30-2: 45$ |  | 12 |  | 10 |  | 1 |
| Total | 74 | 63 | 17 | 17 | 1 | 1 |



Figure 4: AM Drop-Off Queue

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts

Figure 5: PM Pick-Up Queue

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts
### 2.6 Balmer Elementary School Parking Supply and Demand

Nitsch Engineering performed a parking supply and demand count on September 12, 2017. The utilization of the lot was taken at 10:00 AM.

As shown on Figure 6, a total of 91 parking spaces were counted in four sections within the Balmer Elementary School, including 3 of which are accessible spaces. This meets the Architectural Access Board (AAB) Code of Massachusetts Regulations ( 521 CMR ) for the required number of accessible parking spaces. The 3 accessible spaces were not utilized. The overall lot utilization was $74 \%$.

Nitsch Engineering used the Institute of Transportation Engineers (ITE) publication Parking Generation, 4th Edition to estimate the parking demands for the two future school options (Grades 2-4 with 510 students and staff of 49 , or PK-5 with 1110 students and staff of 80 ).

Parking generation rates for the Elementary School were based on Land Use Code (LUC) 520 (Elementary School). We used the Number of Students as the independent variable to base the ITE parking generation rates.

Based on the ITE parking generation rates, the parking demand for a school with a population of 510 is 88 parking spaces.

Based on the ITE parking generation rates, the parking demand for a school with a population of 1110 is 189 parking spaces.


Figure 6: Parking
W. Edward Balmer Elementary School Northbridge, Massachusetts

### 3.1 Crash Data

Nitsch Engineering reviewed the crash data available from MassDOT for the three (3) most recent years available - 2013 to 2015 - for the study intersections. A summary of the crashes, including the severity, and the manner of collision are shown in Table 2.

Table 2-Crash Summary

| Location | Number of Crashes |  |  | Severity |  |  |  | Manner of Collision |  |  |  |  | Percent During |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Year | Total Crashes | Average | PD ${ }^{\text {a }}$ | $\mathrm{Pl}^{\text {b }}$ | NR ${ }^{\text {c }}$ | $\mathrm{F}^{\text {d }}$ | $\mathrm{A}^{\text {e }}$ | REf | $\mathrm{HO}^{\text {² }}$ | Other ${ }^{\text {h }}$ | Incl. <br> Ped- <br> Bike ${ }^{j}$ | Peak Hours ${ }^{k}$ | Wet/lcy Conditi ons |
| N. Main St at Crescent St | 2013 | 0 | 1.00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2014 | 1 |  | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 100\% | 0\% |
|  | 2015 | 2 |  | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 100\% | 0\% |
| Crescent St at Arcade St | 2013 | 0 | 0.67 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2014 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2015 | 2 |  | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 50\% | 50\% |
| Main St at N. Main St | 2013 | 1 | 1.33 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 100\% | 0\% |
|  | 2014 | 1 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0\% | 100\% |
|  | 2015 | 2 |  | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0\% | 50\% |
| Main St at Lake St | 2013 | 0 | 0.33 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\%! | 0\% |
|  | 2014 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2015 | 1 |  | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0\% | 0\% |
| Crescent St at Lake St | 2013 | 0 | 0.00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2014 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2015 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
| Crescent St at School Dr | 2013 | 1 | 0.33 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 100\% | 100\% |
|  | 2014 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
|  | 2015 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0\% | 0\% |
| Total | ALL | 11 | 3.67 | 8 | 3 | 0 | 0 | 6 | 1 | 0 | 4 | 0 | 55\% | 36\% |

${ }^{\text {ap }}$ Property Damage Only; ${ }^{\text {b }}$ Personal Injury Only (non-Fatal Injury); ${ }^{\circ}$ Not Reported; ${ }^{\mathrm{d}}$ Fatality; ${ }^{\mathrm{e}}$ Angle; ${ }^{\mathrm{f}}$ Rear end; ${ }^{\mathrm{q}} \mathrm{Head}$ on; ${ }^{\mathrm{h}}$ Sideswipe, opposite direction;
sideswipe, same direction, single vehicle crash, rear-to-rear, not reported, unknown, etc.; Includes pedestrian or cyclist; koccurred between 7-9am or 4-6pm

A total of 11 crashes were reported within the study areas for the six intersections from 2013 to 2015. In terms of severity, eight of the crashes involved property damage only and three reported personal injury. In terms of manner of collision, six of the crashes were angle collisions, one was a rear-end crash, and four were of other type. Approximately 55\% of the crashes occurred during the peak hours of 7:00 to 9:00 AM or 4:00 to 6:00 PM and $36 \%$ occurred during wet/icy conditions. Analyzing the crash data, as most crashes were of angle or rearend type, the crashes were most likely caused by driver carelessness or inattentiveness.

A factor in determining overall safety of an intersection is to calculate the crash rate by using MassDOT Crash Rate worksheets. The rate at an intersection can be compared to the average for its district and statewide.

The crash data and crash rates are summarized in Appendix A-3.

## EXISTING TRAFFIC CONDITIONS

### 4.1 2017 Traffic Count Data

Automatic Traffic Recorder (ATR) Data
Precision Data Industries, LLC (PDI) of Berlin, Massachusetts was retained by Nitsch Engineering to conduct 48-hour Automatic Traffic Recorder (ATR) vehicle traffic counts throughout the study area; from Tuesday, November 28 to Wednesday, November 29, 2017 (Table 3). A copy of the raw traffic count data is included in Appendix A-1.

Table 3 - Automatic Traffic Recorder (ATR) Summary


## Turning Movement Count (TMC) Data

PDI collected Turning Movement Counts (TMC) data for the study area intersections near the Balmer Elementary School from Tuesday, November 28 to Wednesday, November 29, 2017 from 7:00 AM to 9:00 AM and 1:30 PM to 3:30 PM to capture both morning and afternoon peak periods for the school. The TMC data included bicycle and pedestrian counts. The peak hours within the study area were established as 7:00 AM to 8:00 AM during the weekday morning period and 2:00 PM to 3:00 PM during the afternoon period. The 2017 Existing Traffic Volumes are shown in Figure 7.

## Vehicle Travel Speeds

PDI measured vehicle travel speeds at the ATR locations at the time of the traffic count. The 85th percentile speed, meaning the speed at which $85 \%$ of the vehicles are at or below, is noted because of its importance in determining appropriate roadway speed limits and for calculating required sight distance. The speed data is shown in Table 4.

Table 4 - Vehicle Travel Speeds

| LOCATION | POSTED <br> SPEED <br> $\left(\right.$ MPH $\left.^{\mathbf{a}}\right)$ | 85th <br> PERCENTILE <br> SPEED <br> $\left(\right.$ MPH $\left.^{\mathbf{a}}\right)$ |
| :--- | :---: | :---: |
| North Main Street north of Main Street |  |  |
| $\quad$ Northbound | 30 | 37 |
| Southbound | 30 | 37 |
| Crescent Street and School Driveway |  |  |
| $\quad$ Westbound | 20 | 30 |
| $\quad$ Eastbound | 20 | 28 |
| Main Street west of Water Street | 30 | 41 |
| $\quad$ Westbound |  |  |
| $\quad$ Eastbound | 30 | 42 |
| a $=$ Miles per hour |  |  |
| Note: 85th Percentile Speeds were averaged between the full two days of data collected |  |  |

### 4.2 Seasonal Adjustment

Nitsch Engineering researched data from MassDOT to establish if any seasonal adjustment to the traffic counts was necessary. We used MassDOT's 2013 Weekday Seasonal Adjustment Factors, which is the latest data set available. The data compares monthly traffic volumes from different types of roadways across the Commonwealth to compare the traffic volumes from each individual month to the annual average. During the month of September on urban arterials and collectors in this area, traffic volumes are approximately $7 \%$ higher than an average month. Additionally, the counts were performed while school was in full session, so the traffic counts represent the average condition with respect to traffic within the study area. To be conservative, we made no adjustment to the collected volumes. The Weekday Seasonal Adjustment Factors are included in Appendix A-2.


Figure7: 2017 Existing Volumes
W. Edward Balmer Elementary School Northbridge, Massachusetts

## FUTURE NO-BUILD TRAFFIC CONDITIONS

Nitsch Engineering used the 2017 existing traffic volumes as the baseline for projecting traffic volumes for the chosen seven-year design horizon to the 2024 future no-build condition. To determine the future 2024 No-Build condition, the following steps are included:

- Project existing 2017 traffic volumes seven years in the future to the horizon year (2024) using an annual background traffic growth factor;
- Add traffic volumes associated with any planned developments that may impact the study area; and
- Analyze the study area location to determine future operational statistics.


### 5.1 Background Growth

Nitsch Engineering used the previous 10-year data from MassDOT count station \#3192, located on Hill Street approximately 1.7 miles north of Main Street to calculate the background traffic growth. We used an annual background traffic growth factor of $1 \%$, which is also consistent with recent MassDOT projects in eastern Massachusetts.

### 5.2 Planned Development

Nitsch Engineering contacted the Town of Northbridge to establish if there are any planned development projects in the vicinity of the study that would add additional trips in the near future. Per the Town of Northbridge, there are no new planned developments in the vicinity of Balmer Elementary School.

### 5.3 No-Build Traffic Volumes

The 2024 No-Build Traffic Volumes are shown in Figure 8 and are derived by applying the compounded traffic growth rate of $1 \%$ per year over the seven-year design horizon to project the 2024 traffic volumes.


Figure 8: 2024 Future Volumes - No Build
W. Edward Balmer Elementary School Northbridge, Massachusetts

## 6

 FUTURE CONDITIONSNitsch Engineering performed a design year traffic analysis to compare existing traffic operations with the proposed conditions of constructing a new Balmer Elementary School building and grounds on the existing site based on the two enrollment options of 510 and 1,110 students.

Sketch plans of the four redevelopment options for constructing a new Balmer Elementary School on the existing site are shown in Appendix A-4. The sketch plans show the proposed driveway locations of the school on an existing base map with the site location and outline.

The proposed school options will provide many enhancements to traffic circulations and controls such as a new egress to North Main Street, providing an extended parent queue on site, separation of cars and buses, providing a dedicated delivery access and increased parking.

### 6.1 Proposed School Access and Egress, Circulation, Bus and Parent Pick-Up/Drop-Off

The proposed four development options for constructing a new Balmer Elementary School on the existing site will have identical access and egress points. The new School (regardless of the option) will continue to be accessed from Crescent Street now using two driveways. The access and egress to the school (parental dropoff and pick-up, as well as teachers and staff) will occur from the new driveway opposite Lake Street to the circular driveway at the new main entrance. The existing school driveway will also be used for pre-kindergarten parental drop-off and pick-up as well as access to the parking lots.

The bus and delivery traffic will arrive through the new driveway opposite Lake Street. The bus drop-off/pick-up will occur at the designated bus lane located south of the school.

A new one-way driveway will also allow vehicular egress to North Main Street.

### 6.2 Trip Generation for New School with 510 Student Enrollment Option (Option A)

Nitsch Engineering used the Number of Students as the independent variable to base the trip generation rates. The existing school enrolls 569 students with 55 teachers and staff. The enrollment at the new school will be 510 students for $2^{\text {nd }}$ through $4^{\text {th }}$ grade, and approximately the same number of teachers and staff as there are currently. This means a reduction in school enrollment. To be conservative, we have used the existing enrollment data for our analysis.

Table 5 summarizes vehicle trips generated by the proposed school. The vehicle trips include teachers and staff at the new school.

Table 5 - Proposed Trip Generation - Option A

| TIME PERIOD |  |  |
| :---: | :---: | :---: |
| EXISTING |  |  |
|  | ENTERING | 130 |
|  | EXITING | 92 |
|  | TOTAL | 222 |
| $\mathbf{P M}$ | ENTERING | 81 |
|  | EXITING | 104 |
|  | TOTAL | 185 |

Trip Distribution, and Assignment - Option A

The trips to/from the proposed Balmer Elementary School were distributed and assigned based on the existing travel patterns and logical travel routes, which are based on the existing roadway network both within the Town of Northbridge and the surrounding region.

To properly assess the effect of trips to/from the proposed Balmer Elementary School, the proposed generated drop-off and pick-up trips (Table 6) were assigned to the network. The Trip Distribution Percentages specific to the proposed Balmer Elementary School are shown in Figure 9.

The resultant trip assignment volumes for both the weekday morning and weekday afternoon peak hours were calculated by multiplying the trip distribution by the trip generation from Table 6, and are shown in Figure 10 for the weekday morning and the weekday afternoon peak hours.

Proposed 2024 Build Volumes - Option A

For the proposed Balmer Elementary School, the corresponding trip assignment volumes were balanced based on the proposed access and egress to the school, and redistributed to yield the 2024 Build -Option A Volumes. The 2024 Future Build - Option A Volumes for the proposed Balmer Elementary School are shown in Figure 11.


Figure 9: Trip Distribution - Option A

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts

Figure 10: Trip Assignment - Option A

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts

Figure 11: 2024 Future Volumes - Build Option A
W. Edward Balmer Elementary School Northbridge, Massachusetts

### 6.3 Trip Generation for New School with 1110 Student Enrollment Option (Option B)

Nitsch Engineering used the Number of Students as the independent variable to base the trip generation rates. The existing school enrolls 569 students with 55 teachers and staff. The enrollment at the new school will be 1110 students for pre-kindergarten through $5^{\text {th }}$ grade, and approximately 80 teachers and staff. This means that the school enrollment will double, or grow by approximately $100 \%$. The proposed school trip generation was calculated by increasing the existing trips entering and exiting the school by $100 \%$ to present a conservative analysis of the School Project. Table 6 summarizes vehicle trips generated by the existing and proposed school.

Table 6 - Proposed Trip Generation - Option B

| TIME PERIOD |  | EXISTING | FUTURE <br> VEHICLE TRIPS <br> BASED ON <br> TRAFFIC <br> COUNTS |
| :---: | :---: | :---: | :---: |
| $\mathbf{A M}$ | ENTERING | 130 | 260 |
|  | EXITING | 92 | 184 |
|  | TOTAL | 222 | 444 |
| $\mathbf{P M}$ | ENTERING | 81 | 162 |
|  | EXITING | 104 | 208 |
|  | TOTAL | 185 | 370 |

Table 6 shows that the proposed Balmer Elementary School would result in approximately 222 additional entering and exiting trips during morning drop-off, and approximately 185 additional entering and exiting trips during afternoon pick-up. The increase in vehicle trips includes teachers and staff at the new school.

## Trip Distribution, and Assignment - Option B

The trips to/from the proposed Balmer Elementary School were distributed and assigned based on the existing travel patterns and logical travel routes, which are based on the existing roadway network both within the Town of Northbridge and the surrounding region.

To properly assess the effect of trips to/from the proposed Balmer Elementary School, the proposed generated drop-off and pick-up trips were assigned to the network. The Trip Distribution Percentages specific to the proposed Balmer Elementary School are shown in Figure 12.

The resultant trip assignment volumes for both the weekday morning and weekday afternoon peak hours were calculated by multiplying the trip distribution by the trip generation from Table 7, and are shown in Figure 13 for the weekday morning and the weekday afternoon peak hours.

## Proposed 2024 Build Volumes - Option B

For the proposed Balmer Elementary School, the corresponding trip assignment volumes were balanced based on the proposed access and egress to the school, and added to the 2024 No-Build Volumes to yield the 2024 Build Volumes. The 2024 Future Build - Option B Volumes for the proposed Balmer Elementary School are shown in Figure 14.


Figure 12: Trip Distribution - Option B
W. Edward Balmer Elementary School Northbridge, Massachusetts


Figure 13: Trip Assignment - Option B

## W. Edward Balmer Elementary School

 Northbridge, Massachusetts

Figure 14: 2024 Future Volumes - Build Option B
W. Edward Balmer Elementary School Northbridge, Massachusetts

## 7

## OPERATIONS ANALYSIS

### 7.1 Level of Service Criteria

Level of Service (LOS) is a qualitative measure describing operational conditions within a traffic stream. Six (6) LOS criteria are used to describe the quality of traffic flow for any type of facility controls. LOS A represents the best operating conditions, and LOS-F represents the worst operating conditions. Nitsch Engineering analyzed the levels of service for signalized and unsignalized intersections using Synchro 8 software, which is based on the traffic operational analysis methodology of the Highway Capacity Manual ${ }^{2}$ (HCM). Control delay includes initial deceleration delay, queue move-up time, stopped delay, and final acceleration delay. Table 7 summarizes the relationship between LOS and average control delay for signalized and unsignalized intersections.

Table 7 - Level of Service Criteria

| SIGNALIZED INTERSECTIONS |  | UNSIGNALIZED INTERSECTIONS |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Level of Service | Control Delay (seconds/vehicle) | Level o Volume-to | vice by acity (v/c) | Control Delay (seconds/vehicle) |
|  |  | $\mathrm{v} / \mathrm{c} \leq 1.0$ | $\mathrm{v} / \mathrm{c}$ > 1.0 |  |
| A | 0 to 10 | A | F | 0 to 10 |
| B | $>10$ to 20 | B | F | $>10$ to 15 |
| C | $>20$ to 35 | C | F | $>15$ to 25 |
| D | $>35$ to 55 | D | F | $>25$ to 35 |
| E | $>55$ to 80 | E | F | $>35$ to 50 |
| F | >80 | F | F | >50 |
| Source: 2010 Highway Capacity Manual, Transportation Research Board, Washington D.C. 2010 |  |  |  |  |

### 7.2 Capacity Analysis

Nitsch Engineering performed traffic analyses to evaluate traffic operations for the 2017 Existing Conditions, 2024 No-Build Conditions, and 2024 Build Conditions during the weekday morning and weekday afternoon peak hours at the study intersections. The analyses depict the volume-to-capacity (v/c) ratio, vehicle delay, LOS, and the 50th/95th percentile vehicle queues.

[^2]
### 7.3 2017 Existing Capacity Analysis

Nitsch Engineering analyzed the 2017 Existing Conditions traffic operations at the study intersections based on the existing traffic counts performed by PDI in November 2017. The Level of Service Summary is shown in Table 8. The analysis worksheets are provided in Appendix A-6.

Table 8 - Level of Service Summary - 2017 Existing Conditions

| LOCATION | DIRECTION / MOVEMENT ${ }^{1}$ | WEEKDAY MORNING PEAK HOUR |  |  |  |  | WEEKDAY EVENING PEAK HOUR |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C ${ }^{2}$ | DELAY ${ }^{3}$ | LOS $^{4}$ | $\begin{gathered} 50^{\text {th }} \\ Q^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & Q^{6} \end{aligned}$ | V/C ${ }^{2}$ | DELAY ${ }^{3}$ | LOS $^{4}$ | $\begin{gathered} 50^{\text {th }} \\ \mathrm{Q}^{5} \\ \hline \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & \mathrm{Q}^{6} \\ & \hline \end{aligned}$ |
| Main St at N. Main St | Main St EB-LT | 0.01 | 0.4 | A | - | 1 | 0.04 | 1.4 | A | - | 3 |
|  | Main St WB-TR | 0.21 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | N. Main St SB-LR | 0.51 | 19.4 | C | - | 72 | 0.54 | 24.8 | C | - | 78 |
| Main St at Arcade St | Main St EB-LT | 0.02 | 0.6 | A | - | 1 | 0.01 | 0.3 | A | - | 1 |
|  | Main St WB-TR | 0.20 | 0.0 | A | - | 0 | 0.29 | 0.0 | A | - | 0 |
|  | Arcade St SB-LR | 0.07 | 12.3 | B | - | 6 | 0.05 | 14.4 | B | - | 4 |
| Main St at Lake St | Main St EB-LT | 0.03 | 0.8 | A | - | 2 | 0.01 | 0.4 | A | - | 1 |
|  | Main St WB-TR | 0.26 | 0.0 | A | - | 0 | 0.35 | 0.0 | A | - | 0 |
|  | Lake St SB-LR | 0.67 | 36.7 | E | - | 114 | 0.46 | 28.0 | D | - | 58 |
| N. Main St at Crescent St | Ct St EB-LTR | 0.13 | 12.1 | B | - | 12 | 0.09 | 12.3 | B | - | 7 |
|  | Crescent St WB-LTR | 0.13 | 11.8 | B | - | 11 | 0.12 | 12.0 | B | - | 10 |
|  | N. Main St SB-LTR | 0.02 | 0.9 | A | - | 1 | 0.02 | 1.1 | A | - | 1 |
|  | N. Main St NB-LTR | 0.01 | 0.3 | A | - | 0 | 0.01 | 0.4 | A | - | 1 |
| Crescent St at Arcade St | Crescent St EB-TR | 0.04 | 0.0 | A | - | 0 | 0.04 | 0.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.08 | 9.4 | A | - | 6 | 0.04 | 3.7 | A | - | 3 |
|  | Arcade St NB-LR | 0.11 | 5.6 | A | - | 9 | 0.02 | 9.3 | A | - | 3 |
| Crescent St at School Dr | Crescent St EB-LT | 0.03 | 5.2 | A | - | 2 | 0.01 | 2.9 | A | - | 1 |
|  | Crescent St WB-TR | 0.09 | 0.0 | A | - | 0 | 0.06 | 0.0 | A | - | 0 |
|  | School Dr SB-LR | 0.12 | 10.1 | B | - | 11 | 0.13 | 9.6 | A | - | 11 |
| Crescent St at Lake St | Crescent St EB-TR | 0.09 | 7.1 | A | - | 0 | 0.10 | 7.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.12 | 7.8 | A | - | 0 | 0.03 | 7.4 | A | - | 0 |
|  | Lake St NB-LR | 0.10 | 8.0 | A | - | 0 | 0.09 | 7.8 | A | - | 0 |
| ${ }^{1}$ Volume to Capacity Ratio; ${ }^{2}$ Vehicle Delay, measured in seconds; ${ }^{3}$ Level Of Service; ${ }^{4} 50$ th Percentile Queue (in feet); ${ }^{5} 95$ th Percentile Queue (in feet) based upon 22 feet per vehicle; * = Defacto Left Lane; \# = volume exceeds capacity, queue may be longer; $m=95$ th percentile queue is metered by upstream signal; $\sim$ $=$ Volume exceeds capacity, queue is theoretically infinite |  |  |  |  |  |  |  |  |  |  |  |

### 7.4 2024 No-Build Capacity Analysis

Nitsch Engineering analyzed the 2024 No-Build Conditions traffic operations at the study intersections (See Section 5). The Level of Service Summary is shown in Table 9. The analysis worksheets are provided in Appendix A-6.

Table 9 - Level of Service Summary - 2024 No - Build Conditions

| LOCATION | DIRECTION / MOVEMENT1 | WEEKDAY MORNING PEAK HOUR |  |  |  |  | WEEKDAY EVENING PEAK HOUR |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | V/C ${ }^{2}$ | DELAY ${ }^{3}$ | LOS ${ }^{4}$ | $\begin{gathered} 50^{\text {th }} \\ \mathrm{Q}^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & \mathrm{Q}^{6} \end{aligned}$ | V/C ${ }^{2}$ | DELAY ${ }^{3}$ | LOS ${ }^{4}$ | $\begin{gathered} 50^{\text {th }} \\ \mathrm{Q}^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & \mathrm{Q}^{6} \end{aligned}$ |
| Main St at N. Main St | Main St EB-LT | 0.01 | 0.5 | A | - | 1 | 0.04 | 1.5 | A | - | 3 |
|  | Main St WB-TR | 0.22 | 0.0 | A | - | 0 | 0.33 | 0.0 | A | - | 0 |
|  | N. Main St SB-LR | 0.58 | 22.5 | C | - | 90 | 0.63 | 30.5 | D | - | 101 |
| Main St at Arcade St | Main St EB-LT | 0.02 | 0.6 | A | - | 1 | 0.01 | 0.3 | A | - | 1 |
|  | Main St WB-TR | 0.21 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | Arcade St SB-LR | 0.08 | 12.8 | B | - | 7 | 0.06 | 15.2 | C | - | 5 |
| Main St at Lake St | Main St EB-LT | 0.03 | 0.8 | A | - | 2 | 0.01 | 0.4 | A | - | 1 |
|  | Main St WB-TR | 0.28 | 0.0 | A | - | 0 | 0.35 | 0.0 | A | - | 0 |
|  | Lake St SB-LR | 0.79 | 51.9 | E | - | 157 | 0.46 | 34.5 | D | - | 75 |
| N. Main St at Crescent St | Ct St EB-LTR | 0.15 | 12.7 | B | - | 13 | 0.10 | 12.7 | B | - | 8 |
|  | Crescent St WB-LTR | 0.13 | 12.1 | B | - | 12 | 0.13 | 12.5 | B | - | 11 |
|  | N. Main St SB-LTR | 0.02 | 1.0 | A | - | 1 | 0.02 | 1.1 | A | - | 1 |
|  | N. Main St NB-LTR | 0.01 | 0.3 | A | - | 0 | 0.01 | 0.5 | A | - | 1 |
| Crescent St at Arcade St | Crescent St EB-TR | 0.04 | 0.0 | A | - | 0 | 0.04 | 0.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.12 | 5.7 | A | - | 10 | 0.04 | 3.5 | A | - | 3 |
|  | Arcade St NB-LR | 0.08 | 9.4 | A | - | 7 | 0.02 | 9.3 | A | - | 2 |
| Crescent St at School Dr | Crescent St EB-LT | 0.03 | 4.9 | A | - | 2 | 0.01 | 2.7 | A | - | 1 |
|  | Crescent St WB-TR | 0.9 | 0.0 | A | - | 0 | 0.06 | 0.0 | A | - | 0 |
|  | School Dr SB-LR | 0.13 | 10.2 | B | - | 11 | 0.13 | 9.6 | A | - | 11 |
| Crescent St at Lake St | Crescent St EB-TR | 0.10 | 7.1 | A | - | 0 | 0.11 | 7.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.12 | 7.8 | A | - | 0 | 0.03 | 7.4 | A | - | 0 |
|  | Lake St NB-LR | 0.10 | 8.0 | A | - | 0 | 0.10 | 7.9 | A | - | 0 |
| ${ }^{1}$ Volume to Capacity Ratio; ${ }^{2}$ Vehicle Delay, measured in seconds; ${ }^{3}$ Level Of Service; ${ }^{4} 50^{\text {th }}$ Percentile Queue (in feet); ${ }^{5} 95$ th Percentile Queue (in feet) basedupon 22 feet per vehicle; ${ }^{*}=$ Defacto Left Lane; \# = volume exceeds capacity, queue may be longer; $m=95$ th percentile queue is metered by upstream signal; $\sim$ $=$ Volume exceeds capacity, queue is theoretically infinite |  |  |  |  |  |  |  |  |  |  |  |

### 7.5 Traffic Signal Warrant

To quantify if additional mitigation would be necessary at the proposed school site, based on the student population, and at the access and egress point of Crescent Street at Lake Street, we performed a Signal Warrant Analysis for both Options A and B.

We performed the warrants based on the procedures outlined in the MUTCD 2009 edition. The MUTCD indicates nine (9) separate conditions under which a traffic signal warrant can be met, and they are shown below.

1. Warrant 1: Eight-Hour Vehicular Volume;
2. Warrant 2: Four-Hour Vehicular Volume;
3. Warrant 3: Peak Hour;
4. Warrant 4: Pedestrian Volume;
5. Warrant 5: School Crossing;
6. Warrant 6: Coordinated Signal System;
7. Warrant 7: Crash Experience;
8. Warrant 8: Roadway Network; and
9. Warrant 9: Intersection Near a Grade Crossing.

Given the criteria set forth in the MUTCD and the assumptions above, the intersection of Crescent Street at Lake Street does not meet any warrants for signalization. We believe that the recommendations outlined in Section 8.2 would represent the best return on investment with regards to handling the estimated traffic to and from the new Balmer Elementary School. The Traffic Signal Warrant Analysis is included in Appendix A-5.

### 7.6 Sight Distance

Stopping Sight Distance (SSD) is the length of the roadway ahead that is visible to the driver and should be sufficiently long to enable a vehicle traveling at or near the design speed to stop before reaching a stationary object in its path. Stopping sight distance is the sum of the distance traversed by the vehicle from the instant the driver sights an object necessitating a stop to the instant the brakes are applied and the distance needed to stop the vehicle from the instant brake application begins.

Intersection Sight Distance (ISD) is the length of the leg of the departure sight triangle along the major road in both directions for a vehicle stopped on the minor road waiting to depart. The critical departure sight triangles for the school driveways are for traffic approaching from either the left or right for left turns from the school driveways onto Crescent Street and North Main Street. The methods for determining the sight distances needed by drivers approaching intersections are based on the same principles as stopping sight distance, but incorporate modified assumptions based on observed driver behavior at intersections.

The SSD and ISD values associated with a given design speed are shown in Table 10. The site distance evaluations for the intersection are shown in Table 11.

Table 10 - Sight Distance Criteria

| DESIGN <br> SPEED | DESIGN STOPPING SIGHT <br> DISTANCE VALUE <br> (SSD) | RECOMMENDED INTERSECTION <br> SIGHT DISTANCE VALUE $^{2}$ <br> (ISD) |
| :---: | :---: | :---: |
| $(\mathrm{MPH})$ | (FT) | (FT) |
| 15 | 80 | 170 |
| 20 | 115 | 225 |
| 25 | 155 | 280 |
| 30 | 200 | 335 |
| 35 | 250 | 390 |
| 40 | 305 | 445 |
| 45 | 360 | 500 |
| 50 | 425 | 555 |
| 55 | 495 | 610 |
| 60 | 570 | 665 |
| 65 | 645 | 720 |
| 70 | 730 | 775 |
| 75 | 820 | 830 |
| 80 | 910 | 885 |

Source: A Policy on Geometric Design of Highways and Streets, AASHTO, Washington DC (2011)
${ }^{1}$ Design value based on a grade of less than 3\%, a brake reaction distance predicted on a time of 2.5 seconds and a deceleration rate of $11.2 \mathrm{ft} / \mathrm{s}^{2}$
${ }^{2}$ Recommended value based on Case B1 - a stopped passenger car to turn left onto a two-lane highway with no median and grades $3 \%$ or less

The higher of the posted, or 85th percentile, speed was used to calculate the minimum sight distance to be conservative.

At the intersections of Crescent Street and the school driveways, both the SSD and ISD values meet the minimum values for turning vehicles onto Crescent Street and for both eastbound and westbound traffic on Crescent Street.

At the intersection of North Main Street and the school driveway, both the SSD and ISD values meet the minimum values for turning vehicles onto North Main Street and for both northbound and southbound traffic on North Main Street.

Table 11 - Proposed Sight Distance Evaluation

| INTERSECTION | POSTED SPEED (MPH) | 85th PERCENTILE SPEED (MPH) | MINIMUM (FEET) ${ }^{1,2}$ | MEASURED (FEET) | OBSTRUCTION |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |
| Stopping Sight Distance: |  |  |  |  |  |
| Crescent Street Westbound | 20 | 30 | 368 | >450 |  |
| Intersection Sight Distance: |  |  |  |  |  |
| Looking to the right from Driveway Looking to the left from Driveway | 20 | 30 | 345 | >450 |  |
| Crescent Street at School West Driveway |  |  |  |  |  |
| Stopping Sight Distance: |  |  |  |  |  |
| Crescent Street EastboundCrescent Street Westbound | 20 | 28 | 182 | >450 |  |
|  | 20 | 30 | 368 | >450 |  |
| Intersection Sight Distance: |  |  |  |  |  |
| Looking to the right from Driveway | 20 | 28 | 345 | >450 |  |
| Looking to the left from Driveway | 20 | 30 | 345 | >450 |  |
| North Main Street at School North Driveway |  |  |  |  |  |
| Stopping Sight Distance: |  |  |  |  |  |
| North Main Street Southbound | 25 | 35 | 250 | 600 |  |
| North Main Street Northbound | 25 | 35 | 390 | 466 |  |
| Intersection Sight Distance: |  |  |  |  |  |
| Looking to the right from Driveway | 25 | 35 | 250 | 600 |  |
| Looking to the left from Driveway | 25 | 35 | 390 | 466 |  |
| ${ }^{1}$ Table 3-1. Stopping Sight Distance on Level Roadways |  |  |  |  |  |
| ${ }^{2}$ Table 9-6. Design Intersection Sight Distance - Case B1, Left Turn from Stop |  |  |  |  |  |

### 7.7 2024 Build Capacity Analysis - Option A

Nitsch Engineering analyzed the 2024 Build Conditions traffic operations at the study intersections for reconstruction of a new Balmer Elementary School on the existing site with a student population of 510 (see Section 6). The Level of Service Summary is shown in Table 12. The analysis worksheets are provided in Appendix A-6.

Table 12 - Level of Service Summary - 2024 Build Option A Conditions

| LOCATION | DIRECTION / MOVEMENT ${ }^{1}$ | WEEKDAY MORNING PEAK HOUR |  |  |  |  | WEEKDAY EVENING PEAK HOUR |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\mathrm{V} / \mathrm{C}^{2}$ | DELAY ${ }^{3}$ | $\mathrm{LOS}^{4}$ | $\begin{gathered} 50^{\text {th }} \\ Q^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & Q^{6} \end{aligned}$ | $\mathrm{V} / \mathrm{C}^{2}$ | DELAY ${ }^{3}$ | $\mathrm{LOS}^{4}$ | $\begin{gathered} 50^{\text {th }} \\ Q^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & Q^{6} \end{aligned}$ |
| Main St at N. Main St | Main St EB-LT | 0.01 | 0.4 | A | - | 1 | 0.03 | 1.0 | A | - | 2 |
|  | Main St WB-TR | 0.21 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | N. Main St SB-LR | 0.47 | 18.6 | C | - | 62 | 0.51 | 23.7 | C | - | 70 |
| Main St at Arcade St | Main St EB-LT | 0.02 | 0.6 | A | - | 1 | 0.01 | 0.3 | A | - | 1 |
|  | Main St WB-TR | 0.21 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | Arcade St SB-LR | 0.08 | 12.8 | B | - | 7 | 0.06 | 15.2 | C | - | 5 |
| Main St at Lake St | Main St EB-LT | 0.01 | 0.4 | A | - | 1 | 0.01 | 0.3 | A | - | 1 |
|  | Main St WB-TR | 0.25 | 0.0 | A | - | 0 | 0.33 | 0.0 | A | - | 0 |
|  | Lake St SB-LR | 0.30 | 19.9 | C | - | 31 | 0.34 | 24.0 | C | - | 37 |
| N. Main St at Crescent St | Ct St EB-LTR | 0.07 | 12.4 | B | - | 5 | 0.06 | 12.1 | B | - | 5 |
|  | Crescent St WB-LTR | 0.07 | 11.1 | B | - | 5 | 0.08 | 11.5 | B | - | 7 |
|  | N. Main St SB-LTR | 0.02 | 1.0 | A | - | 1 | 0.01 | 0.9 | A | - | 1 |
|  | N. Main St NB-LTR | 0.00 | 0.0 | A | - | 0 | 0.01 | 0.4 | A | - | 1 |
| Crescent St at Arcade St | Crescent St EB-TR | 0.03 | 0.0 | A | - | 0 | 0.03 | 0.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.03 | 4.2 | A | - | 2 | 0.02 | 2.6 | A | - | 1 |
|  | Arcade St NB-LR | 0.03 | 8.7 | A | - | 3 | 0.02 | 8.7 | A | - | 1 |
| Crescent St at School Dr | Crescent St EB-LT | 0.01 | 2.3 | A | - | 1 | 0.01 | 1.7 | A | - | 1 |
|  | Crescent St WB-TR | 0.05 | 0.0 | A | - | 0 | 0.04 | 0.0 | A | - | 0 |
|  | School Dr SB-LR | 0.08 | 9.6 | A | - | 6 | 0.08 | 9.4 | A | - | 7 |
| Crescent St at New School Dr/Lake St | Crescent St EB-LTR | 0.02 | 2.3 | A | - | 1 | 0.01 | 1.4 | A | - | 1 |
|  | Crescent St WB-LTR | 0.01 | 1.6 | A | - | 1 | 0.0 | 0.6 | A | - | 0 |
|  | School Dr SB-LTR | 0.02 | 9.9 | A | - | 2 | 0.02 | 9.4 | A | - | 1 |
|  | Lake St NB-LTR | 0.11 | 10.8 | B | - | 10 | 0.09 | 9.8 | A | - | 8 |
| N. Main St at School Dr | N. Main St SB-T | 0.13 | 0.0 | A | - | 0 | 0.10 | 0.0 | A | - | 0 |
|  | N. Main St NB-T | 0.14 | 0.0 | A | - | 0 | 0.15 | 0.0 | A | - | 0 |
|  | School Dr WB-LR | 0.03 | 9.6 | A | - | 2 | 0.03 | 9.8 | A | - | 2 |
| $\begin{aligned} & { }^{1} \text { Volume to Capacity Ratio; }{ }^{2} \text { Vehicle Delay, measured in seconds; }{ }^{3} \text { Level Of Service; }{ }^{4} 50^{\text {th }} \text { Percentile Queue (in feet); }{ }^{5} \text { 95th Percentile Queue (in feet) based } \\ & \text { upon } 22 \text { feet per vehicle; }{ }^{*}=\text { Defacto Left Lane; \# = volume exceeds capacity, queue may be longer; } m=95 \text { th percentile queue is metered by upstream signal; } \sim \\ & =\text { Volume exceeds capacity, queue is theoretically infinite } \end{aligned}$ |  |  |  |  |  |  |  |  |  |  |  |

### 7.8 2024 Build Capacity Analysis - Option B

Nitsch Engineering analyzed the 2024 Build Conditions traffic operations at the study intersections for reconstruction of a new Balmer Elementary School on the existing site with a student population of 1110 (see Section 6). The Level of Service Summary is shown in Table 13. The analysis worksheets are provided in Appendix A-6.

Table 13 - Level of Service Summary - 2024 Build Option B Conditions

| LOCATION | DIRECTION / MOVEMENT ${ }^{1}$ | WEEKDAY MORNING PEAK HOUR |  |  |  |  | WEEKDAY EVENING PEAK HOUR |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\mathrm{V} / \mathrm{C}^{2}$ | DELAY ${ }^{3}$ | $\mathrm{LOS}^{4}$ | $\begin{gathered} 50^{\text {th }} \\ Q^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & Q^{6} \end{aligned}$ | $\mathrm{V} / \mathrm{C}^{2}$ | DELAY ${ }^{3}$ | LOS ${ }^{4}$ | $\begin{gathered} 50^{\text {th }} \\ Q^{5} \end{gathered}$ | $\begin{aligned} & 95^{\text {th }} \\ & Q^{6} \end{aligned}$ |
| Main St at N. Main St | Main St EB-LT | 0.02 | 0.9 | A | - | 2 | 0.04 | 1.3 | A | - | 3 |
|  | Main St WB-TR | 0.22 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | N. Main St SB-LR | 0.52 | 21.1 | C | - | 74 | 0.55 | 25.9 | D | - | 78 |
| Main St at Arcade St | Main St EB-LT | 0.03 | 0.8 | A | - | 2 | 0.01 | 0.4 | A | - | 1 |
|  | Main St WB-TR | 0.21 | 0.0 | A | - | 0 | 0.31 | 0.0 | A | - | 0 |
|  | Arcade St SB-LR | 0.10 | 13.3 | B | - | 8 | 0.08 | 15.9 | C | - | 7 |
| Main St at Lake St | Main St EB-LT | 0.02 | 0.7 | A | - | 2 | 0.02 | 0.5 | A | - | 1 |
|  | Main St WB-TR | 0.27 | 0.0 | A | - | 0 | 0.35 | 0.0 | A | - | 0 |
|  | Lake St SB-LR | 0.46 | 25.9 | D | - | 58 | 0.55 | 33.7 | D | - | 77 |
| N. Main St at Crescent St | Crescent St WB-LTR | 0.09 | 13.4 | B | - | 7 | 0.08 | 12.7 | B | - | 6 |
|  | C St EB-LTR | 0.10 | 12.0 | B | - | 8 | 0.12 | 12.0 | B | - | 10 |
|  | N. Main St SB-LTR | 0.04 | 1.7 | A | - | 3 | 0.02 | 1.4 | A | - | 2 |
|  | N. Main St NB-LTR | 0.00 | 0.1 | A | - | 0 | 0.01 | 0.3 | A | - | 1 |
| Crescent St at Arcade St | Crescent St EB-TR | 0.05 | 0.0 | A | - | 0 | 0.04 | 0.0 | A | - | 0 |
|  | Crescent St WB-LT | 0.04 | 4.1 | A | - | 3 | 0.02 | 2.8 | A | - | 2 |
|  | Arcade St NB-LR | 0.04 | 9.0 | A | - | 3 | 0.02 | 8.9 | A | - | 2 |
| Crescent St at School Dr | Crescent St EB-LT | 0.03 | 2.8 | A | - | 2 | 0.02 | 2.2 | A | - | 1 |
|  | Crescent St WB-TR | 0.07 | 0.0 | A | - | 0 | 0.05 | 0.0 | A | - | 0 |
|  | School Dr SB-LR | 0.17 | 10.6 | B | - | 15 | 0.17 | 10.2 | A | - | 16 |
| Crescent St at Lake St | Crescent St EB-LTR | 0.04 | 2.7 | A | - | 3 | 0.02 | 1.7 | A | - | 2 |
|  | Crescent St WB-LTR | 0.01 | 1.2 | A | - | 1 | 0.0 | 0.3 | A | - | 0 |
|  | School Dr SB-LTR | 0.05 | 10.9 | B | - | 4 | 0.04 | 10.0 | B | - | 3 |
|  | Lake St NB-LTR | 0.24 | 13.2 | B | - | 23 | 0.16 | 11.0 | B | - | 14 |
| N. Main St at School Dr | N. Main St SB -T | 0.14 | 0.0 | A | - | 0 | 0.11 | 0.0 | A | - | 0 |
|  | N. Main St NB -T | 0.14 | 0.0 | A | - | 0 | 0.16 | 0.0 | A | - | 0 |
|  | School Dr WB-LR | 0.05 | 9.8 | A | - | 4 | 0.06 | 10.0 | A | - | 5 |
| ${ }^{1}$ Volume to Capacity Ratio; ${ }^{2}$ Vehicle Delay, measured in seconds; ${ }^{3}$ Level Of Service; ${ }^{4} 50{ }^{\text {th }}$ Percentile Queue (in feet); ${ }^{5} 95$ th Percentile Queue (in feet) based upon 22 feet per vehicle; * = Defacto Left Lane; \# = volume exceeds capacity, queue may be longer; $m=95$ th percentile queue is metered by upstream signal; $=$ Volume exceeds capacity, queue is theoretically infinite |  |  |  |  |  |  |  |  |  |  |  |

### 7.9 Parking

The School parking lot, when complete, will provide 116 striped parking spaces for Option A (248 striped parking spaces for Option B) that include 5 accessible spaces (7 accessible spaces for Option B). Based on existing parking utilization, approximately 15 spaces for visitors are planned which may be available for parental parking ( 40 stationary spaces for visitors are planned for Option B). This number exceeds the number of parking spaces required by the Institute of Transportation Engineers (ITE) Parking Generation for land code 520 to facilitate parental parking during drop-off and pick-up times (see Table 14). The curb at the car loop is approximately 1440 linear feet, which can accommodate an additional 72 vehicles. Option B has two drop-off areas: the Grade 1-5 car loop snakes around behind the school and is approximately 1600 linear feet, which can accommodate 80 vehicles; and PK-K drop-off curb in front of the school is approximately 290 linear feet, which can accommodate 15 vehicles.

Table 14 - Proposed Parking Summary

| Option | Parking Spaces <br> Provided | Parking Spaces <br> Required by Institute of <br> Transportation <br> Engineers' Parking <br> Generation for Land Use <br> Code 520 |
| :---: | :---: | :---: |
|  | 116 Striped <br> (5 Accessible) | 95 |
|  | 72 Live Drop-Off |  |
| B | 248 Striped <br> (7 Accessible) |  |
|  | 95 Live Drop-Off |  |

## 8

## CONCLUSIONS AND RECOMMENDATIONS

### 8.1 Conclusions

Nitsch Engineering has been retained by Dore \& Whittier Architects to prepare a qualitative assessment of safety, traffic circulation, and traffic access/egress, associated with the feasibility study and schematic design for the proposed Balmer Elementary School reconstruction project located in Northbridge, Massachusetts.

The Project includes construction of a new Balmer Elementary School building and grounds on the site of the existing school, located at 21 Crescent St in Northbridge.

The following four (4) potential options are being considered:
5. B2, Grades 2-4, New Construction at the Rear of the Site (510 enrollment);
6. C 2, Addition/Renovation of the existing Balmer School, keeping the Academic Wing ( 1,030 enrollment K-5, plus $80 \mathrm{PK}, 1,110$ total);
7. C3, Grades PK-5, New Construction at the Rear of the Site ( 1,030 enrollment K-5, plus 80 PK, 1,110 total); and
8. C5, Grades PK-5, New Construction, Front of the Site ( 1,030 enrollment K-5, plus 80 PK, 1,110 total).

The evaluation was based on the two (2) enrollment options of 510 and 1,110 students, because all four (4) options have identical access and egress points.

The new school (regardless of the option) will continue to be accessed from Crescent Street using two driveways. The access and egress to the school (parental drop-off and pick-up, as well as teachers and staff) will occur from the new driveway opposite Lake Street to the circular driveway at the new main entrance. The existing school driveway will also be used for pre-kindergarten parental drop-off and pick-up as well as access to the parking lots.

The bus and delivery traffic will arrive through the new driveway opposite Lake Street. The bus drop-off/pick-up will occur at the designated bus lane located south of the school.

A new one-way driveway will also allow vehicular egress to North Main Street.

## New Balmer Elementary School with 510 Enrollment Option (Option A)

We examined the future conditions, as well as site circulation with respect to the projected student drop-off and pick-up at the new Balmer Elementary School. This option is not expected to increase traffic volumes at the School and adjacent streets during the weekday morning drop-off and weekday afternoon pick-up, but it will redistribute the existing traffic because of the new driveways providing additional access and egress to the school. The parking lot will contain 116 spaces, of which approximately 15 spaces may be available for parental parking, based on existing parking utilization. The curb at the car loop is approximately 1440 linear feet, which can accommodate an additional 72 vehicles.

## New Balmer Elementary School with 1110 Enrollment Option (Option B)

We examined the future conditions, as well as site circulation with respect to the projected student drop-off and pick-up at the new Balmer Elementary School. This option would result in a doubling of traffic volumes at the school during the weekday morning drop-off and weekday afternoon pick-up, totaling approximately 222 trips
(130 entering and 92 exiting) during the weekday morning drop-off, and approximately 185 trips ( 81 entering and 104 exiting) during the weekday afternoon pick-up. The parking lot will contain 248 spaces, of which approximately 40 spaces may be available for parental parking, based on existing parking utilization. The curbs at the car loops combined can accommodate an additional 95 vehicles.

At the request of Dore \& Whittier Architects, Nitsch ran two scenarios to test the effectiveness of the proposed N Main driveway on overall site traffic efficiency: one model with the drive, and one without. On the model with the driveway, the maximum time delay at the two other exits from the site (onto Crescent Street) was approximately 11 seconds with a queue length of 16 feet (approximately one car length). Without the driveway, the time delay remains approximately 11 seconds, however the queue length increases to 20 feet (approximately two car lengths). This analysis shows that the effect of the proposed third drive on the function of the other two intersections is almost negligible, and the modeled results are certainly within reasonable level-of-service parameters for the two Crescent Street access drives.

Under either scenario, the one existing and two proposed new access drives have sight distances within safe guidelines.

### 8.2 Recommendations

Based on the proposed options for reconstruction of Balmer Elementary School, Nitsch Engineering offers the following recommendations regardless of the chosen option:

- Continue designating Crescent Street as a School Zone under State and local statute, and install the appropriate School Zone signs.
- The sidewalks and accessible ramps along Crescent Street are in acceptable condition. However, pedestrian experience along Crescent Street should be enhanced by improvements to the pedestrian and student crossing signage, and providing advanced warning signing of school entering and exiting traffic.
- Enhance the pedestrian experience along Lake and Arcade Streets by considering improvements to the sidewalks and accessible ramps where needed to accommodate safe walks to school, and providing advanced warning signing of school entering and exiting traffic.
- Reach out to parents via social media to increase safety awareness.


## W. EDWARD BALMER ELEMENTARY SCHOOL

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[^1]:    ${ }^{1}$ Manual on Uniform Traffic Control Devices for Streets and Highways, 2009 Edition, Federal Highway Administration

[^2]:    ${ }^{2}$ Highway Capacity Manual, 2010 Edition, Transportation Research Board (TRB), Washington, D.C.

